

1 **Evaluation of baseflow modelling structure in**
2 **monthly water balance models using 443 Australian**
3 **catchments**

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18 **Abstract:** It is critical for monthly water balance models (MWBM_s) to achieve
19 realistic hydrological modelling of total flow and its components (*i.e.* quick flow and
20 baseflow) in practical application. Various methods have been developed to improve
21 the performances of the three flow components by focusing on calibration procedures.
22 However, the understanding of runoff partitioning structure in MWBM_s for better
23 performances is still very limited, especially whether the storage-discharge
24 relationship is linear or nonlinear at monthly time scale. In this study, model
25 structures for baseflow simulation in 5 widely used MWBM_s are reviewed and
26 modified from a linear storage-discharge relationship to a nonlinear exponential
27 storage-discharge relationship to achieve realistic baseflow simulation in 443
28 catchments from Australia with diverse hydro-climatic conditions. The performances
29 of original and modified models are evaluated and compared through four assessment
30 criteria including Nash-Sutcliffe efficiency (NSE), logarithmic form of NSE
31 (NSE(log)), Pearson correlation coefficient (r) and Bias (B). Basically, the original
32 models with linear storage-discharge relationship perform satisfactorily in simulating
33 total streamflow and quick flow, but degrade remarkably for simulating baseflow with
34 an underestimation of $-60\pm 36\%$ in all study catchments. The modified MWBM_s with
35 nonlinear storage-discharge relationship significantly outperform the original ones for
36 simulating both total streamflow and baseflow. The assessment criteria NSE,
37 NSE(log), r and B of total streamflow improve in $82\pm 4.0\%$ (mean \pm 1 standard
38 deviation of 5 MWBM_s), $72\pm 4.7\%$, $76\pm 4.5\%$ and $51\pm 2.4\%$ study catchments,
39 respectively. The NSE(log) and r of baseflow simulated using the modified MWBM_s
40 have improved in $68\pm 4.6\%$ and $83\pm 4.1\%$ catchments with median improvement of
41 0.17 ± 0.03 and 0.14 ± 0.03 , respectively. It suggests that the exponential nonlinear
42 storage-discharge relationship is more capable for MWBM_s to capture

43 storage-discharge dynamics than the linear one at monthly time scale. This study
44 highlights that, at monthly time scale, the nonlinearity in catchment storage-discharge
45 relationship is a very important factor for MWBMs performance and more studies are
46 required to reveal catchment monthly runoff generation mechanisms.

47 **Keywords:** monthly water balance model; baseflow mechanisms; runoff partitioning
48 structure; storage-discharge relationship

49 **1 Introduction**

50 Monthly water balance models (MWBMs) are important tools for effective water
51 resource management as they have low input requirement, simple model structure and
52 easy to calibrate (Nasseri et al., 2014; Dakhlaoui et al., 2017). Good performance and
53 robustness of MWBMs are fundamental for water resources assessment (Xu and
54 Singh, 1998), streamflow forecasting (Alley, 1984; Schar et al., 2004), climate change
55 impact assessment (Gleick, 1987; Bastola et al., 2011; Chen et al., 2011), and
56 snowmelt runoff simulation (Xu et al., 1996; Rezaeianzadeh et al., 2013). Lumped
57 MWBMs tend to oversimplify the complexity of hydrological processes, which casts
58 doubt on their capacity to predict seasonal flows under various climate conditions
59 (Dakhlaoui et al., 2017; Hamel et al., 2017). In the majority of widely used MWBMs,
60 such as the Dynamic Water Balance Model (DWBM) (Zhang et al., 2008), Belgium
61 Model (VUB) (Vandewiele et al., 1992), Time Variant Gain Model (TVGM) (Xia et
62 al., 1997), WatBal Model (WM) (Leaf and Brink, 1973) and Schaake Model (SM)
63 (Schaake, 1990), runoff generation process consists of quick flow generation and
64 baseflow generation, referred as runoff partitioning structure. To improve the
65 performance of MWBMs for simulating total streamflow, Bai et al. (2015) modified
66 the evapotranspiration equations but the performances of total streamflow have no
67 significant improvement. The improvement of total flow performance in MWBMs
68 should focus on runoff generation mechanisms rather than actual evapotranspiration
69 process (Vandewiele et al., 1992; Bai et al., 2015). However, studies on the

70 deficiencies in MWBMs are limited and it is important to assess the model structure
71 for runoff generations, especially the baseflow that is of critical importance for water
72 resource management and ecosystem health.

73 Evaluation of hydrological behaviours extracted from total streamflow can guide
74 model improvements in a meaningful way (Gupta et al., 2008; Yilmaz et al., 2008)
75 and achieve realistic hydrological simulation (Duan et al., 2006; McMillan, 2020). For
76 MWBMs with runoff partitioning structure, performances of quick flow and baseflow
77 provide new insight of internal model behaviour, which can be directly used to detect
78 the deficiency of runoff partitioning structure (Shafii et al., 2019). The ideal structure
79 of MWBMs is supposed to achieve realistic representation of the real world, namely
80 keeping acceptably accurate simulations of not only total streamflow but also quick
81 flow and baseflow (Gupta et al., 2008; Euser et al., 2013; Khatami et al., 2019).
82 However, good performance of total streamflow does not necessarily mean internal
83 model processes are correct as it may be achieved under insufficient parameterization
84 constraints and improper conceptualization of hydrological processes in real-world
85 systems (Hrachowitz et al., 2014). Dynamics in quick flow and baseflow can be
86 improperly simulated due to the weaknesses in calibration procedures (Beven, 1993;
87 Bai et al., 2018) and structural inadequacy (Shafii et al., 2017). Many approaches
88 have been proposed to improve calibration procedures such as multi-objective
89 optimization framework (Shafii and Tolson, 2015; Kelleher et al., 2017; He et al.,
90 2018; Larabi et al., 2018; Schuite et al., 2019), temporal variation of parameters

91 (Deng et al., 2018; Xiong et al., 2019) and alternative calibration criteria (Gupta et al.,
92 2009; Larabi et al., 2018; Fowler et al., 2018a). In these studies, observed signals of
93 quick flow and baseflow have been incorporated into multi-objective optimization
94 framework, which results in reliable performance of quick flow and baseflow.
95 However, studies to achieve realistic simulation of total streamflow, quick flow and
96 baseflow through evaluating and developing runoff partitioning structure in MWBMs
97 are still very limited, especially the baseflow modelling structure (Westra et al., 2014;
98 Fowler et al., 2018b).

99 Although most MWBMs use a linear storage-discharge relationship to describe
100 storage-discharge dynamics, the storage-discharge relationship at monthly time scale
101 is still unclear. Catchment storage-discharge relationship is traditionally established at
102 event or daily time scales in previous studies and is rarely investigated at monthly
103 time scale. At event or daily time scale, there has been an on-going discussion for
104 decades that whether the storage-discharge relationship is linear or nonlinear (Moore,
105 1997; Wittenberg, 1999; Lee, 2007). Various linear and nonlinear storage-discharge
106 relationships have been developed (Stoelzle et al., 2015) via mathematical derivation
107 (Duffy, 1996), recession analysis (Chapman, 1999; Aksoy and Wittenberg, 2011;
108 Cheng et al., 2016) and hydrological modelling analysis (Fenicia et al., 2006;
109 Markovic and Koch, 2015). At short time scale, the linear storage-discharge
110 relationship has solid physical basis, which conceptualize moisture storage as a
111 straight-sided bucket with a hole at the bottom (Beven, 2001). The moisture storage at

112 short time scale is assumed to be replenished by previous rainfall events and the
113 recharge from the current rainfall event is typically neglected (Buttle, 1994;
114 Wittenberg, 1999). However, at monthly time scale, the mechanism of baseflow
115 generation is different because the recharge to soil water storage from precipitation at
116 the current month must be considered (Lindstrom et al., 1997; Hrachowitz et al.,
117 2014). Therefore, the linear storage-discharge relationships based on straight-sided
118 bucket conceptualization in MWBMs have to be carefully investigated.

119 To evaluate the baseflow modelling structure in monthly water balance models, 5
120 widely used MWBMs (the DWBM, VUB, TVGM, WM and SM) with both quick
121 flow and baseflow generation processes are selected. The 5 selected MWBMs all
122 adopt a linear storage-discharge relationship to describe baseflow generation
123 mechanism at monthly time scale. Observed hydroclimatic data from 443 catchments
124 across Australia with a wide range of climatic and physiographical conditions are
125 collected to test the performance of models with two different types of catchment
126 storage-discharge relationships. First, the performances of these 5 MWBMs in their
127 original form (*i.e.* with a linear storage-discharge relationship) are assessed in terms
128 of their capability for simulating total streamflow, quick flow and baseflow. Then, the
129 linear storage-discharge relationships in all selected MWBMs are replaced with a
130 nonlinear exponential relationship proposed by Peters and Aulenbach (2011)
131 (hereafter denoted as PA11). The performances of modified MWBMs are evaluated
132 for simulating total flow and baseflow. The primary objectives of this study are: (1) to

133 diagnose the performance in runoff partitioning structure of 5 widely applied
134 MWBMs with both quick flow and baseflow generation processes; (2) to examine the
135 influences of nonlinear storage-discharge relationship on the capability of MWBMs
136 for simulating total streamflow; (3) to examine the ability of nonlinear
137 storage-discharge relationship for MWBMs to achieve realistic hydrological
138 modelling performance in terms of baseflow.

139 **2 Study catchments and data**

140 Daily streamflow of 443 un-nested catchments in Australia with minimum
141 human interferences (without dams, intensive irrigation and land use change) are
142 collected to test the performance of MWBMs (Figure 1). All these catchments are part
143 of the Australia unregulated catchment dataset (Zhang et al., 2013). The collected
144 streamflow, precipitation and potential evapotranspiration data span over the period of
145 1975-2012. All the catchments have a minimum length of 20-year records with at
146 least 10-year continuous records and less than 10% missing daily data in total. The
147 drainage area ranges from the order of 10 to 10000 km². Based on the Köppen-Geiger
148 climate classification map produced by Kottek et al. (2006), the 443 catchments cover
149 all the 5 distinct climatic zones in Australia including tropics, arid, equiseasonal-hot,
150 equiseasonal-warm and winter rainfall dominant (see Figure 1). The number of the
151 catchments within tropics, arid, equiseasonal-hot, equiseasonal-warm and winter
152 rainfall climate zones is 56, 50, 105, 171 and 61, respectively. The average
153 precipitation of all catchments is 958±421 (mean ± standard deviation), potential

154 evapotranspiration is 1411 ± 294 , aridity index is 1.76 ± 1.01 , runoff coefficient is
155 0.19 ± 0.15 and baseflow index is 0.28 ± 0.15 . The coefficient of variation (CV) of
156 monthly precipitation, defined as the ratio of standard deviation (σ) to mean (μ)
157 monthly precipitations, is 0.89 ± 0.31 . The CV of monthly runoff, representing the
158 integrated effects of geological and climatic characteristics on catchments runoff, is
159 2.36 ± 1.33 (ranges see [Table 1](#)).

160 **3 Methodology**

161 **3.1 Separation of quick flow and baseflow**

162 Daily observed total streamflow is separated into daily quick flow and daily
163 baseflow using the Lyne-Hollick (denoted as LH) method ([Lyne and Hollick, 1979](#)).
164 The LH method is adopted in this study not only because it has been widely applied
165 worldwide ([Ahiablame et al., 2013](#)), but also due the reason that it yields practically
166 equivalent results as other complex physical methods ([Cheng et al., 2012; Zhang et al.,](#)
167 [2017](#)). The principle of this separation method is based on signal processing theory.
168 According to the high frequency characteristic of quick flow, the filter equation for
169 quick flow is expressed as:

$$170 \quad Q_{d(i)} = f_1 Q_{d(i-1)} + \frac{1+f_1}{2} (Q_{(i)} - Q_{(i-1)}) \quad (1)$$

171 where Q is total streamflow (mm d^{-1}); Q_d is quick flow (mm d^{-1}); i is the index of
172 time step; and f_1 is the filter parameter (unit of d^{-1}), which is also called the
173 recession constant. Baseflow Q_b can be calculated subsequently by:

$$Q_{b(i)} = \begin{cases} Q_{(i)} - Q_{d(i)}, & Q_{(i)} > Q_{d(i)} \\ 0 & , \quad Q_{(i)} \leq Q_{d(i)} \end{cases} \quad (2)$$

175 Here the digital filter is applied in a traditional way, *i.e.* baseflow is separated
 176 from total flow with three passes (forward, backward and forward) and the filter
 177 parameter (recession constant) f_1 is set to 0.925 as suggested by [Nathan and](#)
 178 [McMahon \(1990\)](#). Separated daily quick flow and baseflow are aggregated to monthly
 179 values and are taken as the observed monthly quick flow and baseflow for evaluating
 180 model performance.

181 **3.2 Descriptions of the MWBMs**

182 In this study, 5 widely applied monthly water balance models with both quick
 183 flow and baseflow generation process are chosen to assess the runoff partitioning
 184 structure in these MWBMs. They are the Dynamic Water Balance Model (DWBM)
 185 ([Zhang et al., 2008](#)), the Belgium Model (VUB) ([Vandewiele et al., 1992](#)), the Time
 186 Variant Gain Model (TVGM) ([Xia et al., 2005](#)), the WatBal Model (WM) ([Leaf and](#)
 187 [Brink, 1973](#)) and the Schaake Model (SM) ([Schaake, 1990](#)). The general water
 188 balance equation of all these models can be expressed as:

$$189 \quad (S(t) - S(t - 1))/\Delta t = P(t) - E_a(t) - Q_d(t) - Q_b(t) \quad (3)$$

190 where $S(t-1)$ and $S(t)$ are the soil moisture storage (unit of mm) at the beginning and
 191 end of the time interval t , respectively; $P(t)$, $E_a(t)$, $Q_d(t)$, $Q_b(t)$ are precipitation,
 192 actual evapotranspiration, quick flow and baseflow, respectively. Unit of $P(t)$, $E_a(t)$,
 193 $Q_d(t)$ and $Q_b(t)$ is mm month^{-1} . For a given time step (*i.e.* month), Δt is equal to

194 1. Basically, all the selected 5 MWBMs have similar conceptual structure for
195 estimating actual evapotranspiration (E_a), quick flow (Q_d) and baseflow (Q_b). The
196 only differences in model structure are the number of water storages and whether
197 equations for estimating different components of water budget are linear or not. The
198 structure of 5 MWBMs are shown in [Figure 2](#). Equations for simulating actual
199 evapotranspiration, quick flow and baseflow of the 5 models are summarized in [Table](#)
200 [2](#). The symbols w_1-w_{17} (see [Figure 2](#) and [Table 2](#)) represent serial numbers of the
201 equations of 5 original MWBMs. Detailed descriptions of all the 5 models are
202 provided in the Appendix. Major similarities and differences are briefly summarized
203 here.

204 Four of the 5 model (*i.e.* VUB, TVGM, WM and SM) have only one soil water
205 storage to estimate Q_d , E_a and Q_b . Only the DWBM has two soil water storages (*i.e.*
206 upper soil water storage (S) and lower groundwater storage (G)), and soil water in
207 upper storage can recharge to the lower groundwater storage. In the DWBM, Q_d and
208 E_a are generated from the upper soil water storage, while Q_b is generated from lower
209 groundwater storage.

210 [Table 3](#) summarizes whether equations for estimating quick flow, baseflow and
211 evapotranspiration of different MWBMs are linear or not. For quick runoff (Q_d), it is
212 simulated as a nonlinear function of precipitation and amount of soil water in all 5
213 models. With respect to baseflow (Q_b), all the selected models estimate Q_b using a
214 linear storage-discharge relationship. Regarding to the actual evapotranspiration (E_a),

215 all selected models estimate E_a as a function of soil moisture and potential
216 evapotranspiration. The SM and WM adopt a simple linear function to estimate
217 monthly E_a , while other models use nonlinear functions.

218 **3.3 Modification of baseflow generation mechanism**

219 [Table 4](#) shows the modification of the linear storage-discharge relationship in 5
220 original MWBMs to a nonlinear storage-discharge relationship proposed by [Peters](#)
221 [and Aulenbach \(2011\)](#) (hereafter denoted as PA11) with parameterization and
222 equations for estimating E_a , Q_d and S are all kept unchanged. The PA11 can be
223 written as:

$$224 \quad W(t) = S(t - 1) + P(t) \quad (22)$$

$$225 \quad Q_b(t) = e^{(W(t)-b)/m} \quad (23)$$

226 where $Q_b(t)$ is baseflow at time step t ; $W(t)$ is the available water to generate
227 baseflow; $S(t - 1)$ is catchment soil moisture storage at time step $t - 1$; $P(t)$ is
228 precipitation at time step t ; b and m are constants to be calibrated. Parameter m
229 determines the nonlinear variability between $W(t)$ and $Q_b(t)$. Parameter b mainly
230 influences the magnitude of $Q_b(t)$.

231 In the PA11, soil moisture storage S includes both shallow soil water storage and
232 groundwater storage as defined by [Aulenbach and Peters \(2018\)](#) and thus all MWBMs
233 are supposed to have only one moisture storage. Four of the five study MWBMs
234 (except the DWBM) have only one water storage and thus the storage structure of

235 these four models are kept the same. In both original and modified forms of these four
236 models, moisture storage supplies water for E_a , Q_d and Q_b . Storage structure of the
237 DWBM model is changed to replace the linear storage-discharge relationship to a
238 nonlinear one. The original DWBM with two water storages is restructured to one
239 storage to incorporate the PA11. For the original DWBM, upper storage (*i.e.* soil
240 storage S) supplies water for actual evapotranspiration (E_a) and discharge (R) to the
241 lower storage (*i.e.* groundwater storage G), from which baseflow (Q_b) is generated.
242 While in the modified DWBM (denoted as DWBM_{mod}), both E_a and Q_b are
243 generated from the same united soil storage. Meanwhile, in all the models, equations
244 for estimation Q_d , E_a and S are all kept unchanged, which are shown in [Table 2](#).

245 **3.4 Parameter estimation and model evaluation**

246 In this study, parameters are calibrated using an automatic optimization technique,
247 Genetic Algorithm (GA) ([Grefenstette et al., 1986](#)). Five criteria are selected to assess
248 model performance including the Nash-Sutcliffe efficiency (NSE, ([Nash and Sutcliffe,](#)
249 [1970](#))), logarithmic form of NSE (NSE(log)), Pearson correlation coefficient (r), Bias
250 Score (BS) ([Wang et al., 2011](#)) and Bias (B). The objective function (F_{opt}), which is
251 used to optimize parameter sets, combines four criteria (NSE, NSE(log), r and BS)
252 that can minimize both systematic (*e.g.* BS) and dynamic error (*e.g.* NSE and
253 NSE(log)) between the simulated and observed high and low flows ([Krause et al.,](#)
254 [2005](#)). The mathematic formulations of the five criteria and F_{opt} are as follows:

255
$$NSE = 1 - \frac{\sum_{t=1}^n (Q_{sim}(t) - Q_{obs}(t))^2}{\sum_{t=1}^n (Q_{obs}(t) - \overline{Q_{obs}})^2} \quad (24)$$

256
$$NSE(\log) = 1 - \frac{\sum_{t=1}^n (\ln(Q_{sim}(t)) - \ln(Q_{obs}(t)))^2}{\sum_{t=1}^n (\ln(Q_{obs}(t)) - \ln(\overline{Q_{obs}}))^2} \quad (25)$$

257
$$r = \frac{\sum_{t=1}^n (Q_{sim}(t) - \overline{Q_{sim}})(Q_{obs}(t) - \overline{Q_{obs}})}{\sqrt{\sum_{t=1}^n (Q_{sim}(t) - \overline{Q_{sim}})^2 \sum_{t=1}^n (Q_{obs}(t) - \overline{Q_{obs}})^2}} \quad (26)$$

258
$$BS = 1 - \left(\max\left(\frac{\overline{Q_{sim}}}{\overline{Q_{obs}}}, \frac{\overline{Q_{obs}}}{\overline{Q_{sim}}}\right) - 1 \right)^2 \quad (27)$$

259
$$B = 1 - \text{abs}\left(\frac{\overline{Q_{sim}} - \overline{Q_{obs}}}{\overline{Q_{obs}}}\right) \quad (28)$$

260
$$F_{opt} = (NSE + NSE(\log) + r + BS)/4 \quad (29)$$

261
$$F_{avg} = (NSE + NSE(\log) + r + B)/4 \quad (30)$$

262 where $Q_{sim}(t)$ and $Q_{obs}(t)$ are the simulated and observed flow at time step t ,
 263 respectively; variables with overbar denote average value; n is the number of months
 264 during the study period.

265 In this study, parameters of both original and modified models are calibrated
 266 against observed total streamflow only by maximizing the value of F_{opt} . Separated
 267 quick runoff and baseflow are not used to calibrate parameters but are used to assess
 268 the capability of original and modified MWBMs for simulating different flow
 269 components. Model performances for simulating total flow, baseflow and quick flow
 270 are evaluated by NSE, NSE(log), r , B and F_{avg} . The BS is much more sensitive to
 271 very poorly simulated flows than B . The BS is used in F_{opt} for parameter
 272 optimization to guarantee much more suitable parameters for baseflow simulation that
 273 can be identified. When calibrated against total flow only, total volume of baseflow

274 may not be well simulated in a few catchments, which can result in large negative
275 values of F_{opt} and makes comparison and visualization of results of all the
276 catchments very difficult. Therefore, bias (B), which can also measure the systematic
277 error as the BS , is chosen to calculate performance index F_{avg} for evaluation.

278 The NSE, NSE(log), BS and B can vary from $-\infty$ to 1.0 and r can vary from
279 -1.0 to 1.0. The closer the NSE, NSE(log), r , BS and B approach 1.0, the better the
280 model performs. The NSE=1.0 or NSE(log)=1.0 means simulated flows are exactly
281 the same as observed flows in every time step. The $r=1$ means the predicted flows
282 show a complete linear relationship with the observed flows. The $BS=1.0$ or $B=1.0$
283 means the volume of simulated and observed flows are the same and there is no
284 systematic error. For the evaluation of baseflow performance, logarithmic form of
285 NSE (NSE(log)) and correlation coefficient (r) are more suitable than NSE and B
286 because baseflow is typically a few orders of magnitude smaller than total flow and
287 quick flow, which will be discussed in section 5.1.

288 **4 Results**

289 **4.1 Performances of the original MWBMs**

290 Performances of the 5 MWBMs in their original forms for estimating total flow
291 (Q), quick flow (Q_d) and baseflow (Q_b) in all the 443 catchments are shown in [Figure](#)
292 [3](#). Basically, all the MWBMs perform satisfactorily in simulating total streamflow and
293 quick flow. However, all the models perform poorly in simulating baseflow.

294 As for the performance of total flow (Q), the median F_{avg} of all the models is
295 larger than 0.68 with a range of 0.68 ~ 0.77 (see [Table 5](#)). The DWBM and VUB have
296 the best performance with median $F_{avg}=0.77$, followed by the TVGM (0.71), SM
297 (0.71), and WM (0.68). The inter-quantile range (IQR, *i.e.* range between 75th and the
298 25th percentiles) of F_{avg} varies from 0.11 to 0.19. The VUB model is the most robust
299 model with an IQR of 0.11, followed by the DWBM (0.12), SM (0.14), WM (0.18),
300 and TVGM (0.19). The median F_{avg} of all the five MWBMs is quite far from the
301 perfect match between the observed and simulated total flow, *i.e.* both $F_{avg} = 1.0$
302 and $IQR = 0.0$.

303 Regarding to the performance of quick flow (Q_d), the median F_{avg} of all the
304 models is higher than 0.52 with a range of 0.52 ~ 0.63. The VUB has the best
305 performance with median $F_{avg}=0.63$, followed by the SM (0.57), DWBM (0.56),
306 TVGM (0.55), and WM (0.52). The IQR of F_{avg} is smaller than 0.27 with a range
307 from 0.15 to 0.27. The TVGM is the most robust model with an IQR of 0.15,
308 followed by the WM (0.16), VUB (0.17), SM (0.19), and DWBM (0.27). The median
309 F_{avg} and IQR of quick flow are roughly as good as those of total flow for all the 5
310 MWBMs, which means the parameterization schemes of the quick flow are accurate
311 in all these models.

312 With respect to the performance of baseflow (Q_b), the median F_{avg} of all the
313 models is smaller than 0.39 with a range of 0.11 ~ 0.39. The SM has the best
314 performance with median $F_{avg}=0.39$, followed by the DWBM (0.30), VUB (0.12),

315 WM (0.12), and TVGM (0.11). The IQR of F_{avg} ranges from 0.21 to 0.42. The VUB
316 is the most robust model with IQR equal to 0.21, followed by the SM (0.27), TVGM
317 (0.33), DWBM (0.39), and WM (0.42). For baseflow, the median F_{avg} is almost
318 three times smaller and the IQR is about twice wider than those of total flow.

319 Comparison of observed and simulated monthly baseflow by all the 5 MWBMs
320 in their original forms in the 443 catchments over the study period are shown in
321 [Figure 4](#). The baseflow is significantly underestimated by all the models about
322 $-60\pm36\%$. The median Pearson correlation coefficient (r) between baseflow
323 estimated by the 5 original MWBMs and observed baseflow is smaller than 0.62 with
324 a range of 0.48 ~ 0.62. These results indicate the linear storage-discharge relationships
325 in all the 5 MWBMs are not appropriate for baseflow simulation. Therefore, model
326 structure for simulating baseflow in these MWBMs has to be modified to improve
327 model performances of both baseflow and total streamflow.

328 **4.2 Performances of the modified MWBMs in simulating total** 329 **streamflow**

330 [Figure 5](#) shows total flow performances of the 5 MWBMs together in their
331 original and modified forms across 443 study catchments. The modified models
332 outperform original models clearly in terms of the NSE ([Figure 5a](#)) and NSE(log)
333 ([Figure 5b](#)), marginally in terms of the r ([Figure 5c](#)) and B ([Figure 5d](#)). Performances
334 of the modified models in terms of the objective values of 4 evaluation indices using

335 box-plot are compared with original models in [Figure 3](#) as well in all the study
336 catchments. [Figure 6](#) shows the changes in model performance between modified and
337 original MWBMs individually in simulating total streamflow. The modified MWBMs
338 outperform the original models on total streamflow in terms of the percentages of
339 catchments that have a better performance ([Figure 6a](#)), median increased value
340 ([Figure 6b](#)) and change in IQR ([Figure 6c](#)) of all study catchments in four different
341 criteria (*i.e.* NSE, NSE(log), r and B).

342 For the criterion of NSE, all modified MWBMs have higher NSE in most study
343 catchments. All the models show smaller IQR compared with the original models,
344 except for the VUB model (see [Table 6](#)). The modified models have higher NSE in
345 $82\pm 4.0\%$ catchments with a range of 72% ~ 93%. The WM has the largest proportion
346 of catchments that is improved (93%), followed by the VUB (85%), TVGM (82%),
347 DWBM (77%) and SM (72%). The median improved NSE for all the study
348 catchments is 0.03 ± 0.007 with a range of 0.01 ~ 0.05. The WM has the largest median
349 improved NSE (0.05), followed by the SM (0.03), DWBM (0.03), TVGM (0.02), and
350 VUB (0.01). The IQR of NSE reduces about 0.02 ± 0.02 with a range of -0.05 to 0.06.
351 The DWBM has the largest reduction of IQR (0.06), followed by the SM (0.05), WM
352 (0.03), TVGM (0.02) and VUB (-0.05).

353 All the 5 modified models have higher NSE(log) in most study catchments and
354 different change of IQR compared with those of the original models. Compared with
355 original models, modified models are better in simulating total streamflow in $72\pm 4.7\%$

356 catchments with a range of 61% ~ 81% in term of NSE(log). The TVGM has the
357 largest proportion of catchments that is improved (81%), followed by the DWBM
358 (79%), WM (77%), SM (63%) and VUB (61%). The median improved NSE(log) for
359 all the study catchments is 0.03 ± 0.008 with a range of 0.01 ~ 0.05. The DWBM has
360 the largest median improved NSE(log) (0.05), followed by the WM (0.04), TVGM
361 (0.04), SM (0.02) and VUB (0.01). The IQR of NSE(log) has reduced about
362 0.002 ± 0.02 with a range of -0.04 to 0.06. The DWBM has the largest reduction of
363 IQR (0.06), followed by the SM (0.03), WM (0.00), TVGM (-0.04) and VUB
364 (-0.04).

365 For the criterion of r , the 5 modified models also have marginal higher r in most
366 study catchments and smaller IQR. The modified models perform better for
367 simulating total streamflow in $76 \pm 4.5\%$ catchments with a range of 61% ~ 86% in
368 terms of r . The WM has the largest proportion of catchments that is improved (86%),
369 followed by the VUB (79%), DWBM (79%), TVGM (77%) and SM (61%). The
370 median improved r for all the study catchments is 0.01 ± 0.002 with a range of 0.00 ~
371 0.01. Except for the SM (0.00), the median improved r of other models values 0.01.
372 The IQR of r has reduced about 0.02 ± 0.01 with a range of 0.00 to 0.04. The DWBM
373 has the largest reduction of IQR (0.04), followed by the SM (0.02), WM (0.02),
374 TVGM (0.01) and VUB (0.00).

375 For the criterion of B , the 5 modified models have marginal improvement of B
376 and marginal reduction of IQR. The modified models have improvement in $51 \pm 2.4\%$

377 study catchments. The median improved B of the 5 modified model is 0.002 ± 0.002
378 and mean reduction of IQR is 0.004 ± 0.003 .

379 In summary, compared to original models, the NSE, NSE(log), r and B of
380 modified models are better for the simulation of total streamflow in $82\pm 4.0\%$,
381 $72\pm 4.7\%$, $76\pm 4.5\%$ and $51\pm 2.4\%$ study catchments, respectively. The median
382 improved NSE, NSE(log), r and B are 0.03 ± 0.007 , 0.03 ± 0.008 , 0.01 ± 0.002 and
383 0.002 ± 0.002 , respectively. The IQR of NSE, NSE(log), r and B have reduced about
384 0.02 ± 0.02 , 0.002 ± 0.02 , 0.02 ± 0.01 , 0.004 ± 0.003 , respectively. Increase in model
385 performance and decrease in IQR suggest that MWBMs became more reliable and
386 robust by replacing the linear storage-discharge relationship with an exponential
387 nonlinear (*i.e.* PA11) relationship.

388 **4.3 Performance of modified models in simulating baseflow**

389 **Figure 7** shows the comparison of baseflow performance of the 5 MWBMs
390 together between their original and modified forms across 443 study catchments. The
391 modified models outperform original models clearly on the NSE (when $NSE > 0$)
392 (**Figure 7a**), NSE(log) (**Figure 7b**) and r (**Figure 7c**). Both original and modified
393 MWBMs have poor NSE with nearly 70% catchments smaller than 0. **Figure 8** shows
394 the changes in model performances between modified and original MWBMs for
395 simulating baseflow individually. Basically, the modified MWBMs outperform the
396 original models in terms of NSE (log) and r , but underperform original models in

397 terms of NSE (all catchments) and B . All 5 modified MWBMs have much higher r
398 and NSE(log) in $83\pm 4.1\%$ and $68\pm 4.6\%$ study catchments comparing with those of
399 the original models, respectively (Figure 8a, Table 7). The median improved r and
400 NSE(log) of all study catchments are 0.14 ± 0.03 and 0.17 ± 0.03 , respectively (Figure
401 8b). Change in IQR of r is marginal (0.03 ± 0.04 , Figure 8c). The IQR of NSE(log) has
402 reduced significantly about 0.12 ± 0.08 . For the criteria of NSE and B , simulated
403 baseflow using modified models is better than that using original ones in about half
404 study catchments. For NSE, the modified MWBMs perform better in $41\pm 7.2\%$ but
405 worse in $59\pm 7.2\%$ catchments. For B , the modified MWBMs perform better and
406 worse in $46\pm 8.3\%$ and $54\pm 8.3\%$ catchments, respectively. The median improved NSE
407 and B of all study catchments are -0.08 ± 0.05 and -0.04 ± 0.06 , respectively. The
408 change of IQR of NSE and B are 0.99 ± 0.57 and 0.27 ± 0.06 , respectively.

409 The increased NSE(log) and r suggest general improvement of baseflow
410 simulation using nonlinear baseflow modelling structure because NSE(log) is more
411 suitable than NSE to evaluate the performance of baseflow and r is the most direct
412 criterion to evaluate whether the storage-discharge relationship is exponential
413 nonlinear or not. According to the equation (31), the much higher r (*i.e.* higher A in
414 Eq.31) of the modified MWBMs provides a precondition of model structure for
415 MWBMs to have higher NSE on baseflow simulation. It directly indicates that the
416 nonlinear relationship (*i.e.* PA11) is better than the linear relationship to capture
417 catchment storage-discharge dynamics at monthly time scale.

418 **5 Discussion**

419 **5.1 Characteristics of different criterion and their suitability for** 420 **evaluation the performance of baseflow**

421 Every criterion has advantages and disadvantages in quantifying the agreement
422 between observed and simulated flows. The Nash-Sutcliffe efficiency (NSE) proposed
423 by [Nash and Sutcliffe \(1970\)](#) is a widely used assessment criterion. The NSE is
424 largely a dynamic indicator and it is very sensitive to high flows and is insensitive to
425 low flows because of its squared formulation ([Legates and McCabe Jr., 1999](#)). To
426 compensate the disadvantage of NSE, NSE(log) is used to give more weights on low
427 flows in the performance assessment. Pearson correlation coefficient (r) measures the
428 co-variability of the simulated and observed flows, which describes how much of the
429 dispersion in observed flows is explained by the simulated flows. The BS and B are
430 employed to measure symmetric error between simulated and observed flows.

431 The four different criteria (*i.e.* NSE, NSE(log), r and B) provide useful insight
432 into basic characteristics of simulation performance. For the evaluation of baseflow
433 performance, logarithmic form of NSE (*i.e.* NSE(log)) and correlation coefficient (r)
434 are more important than NSE and B . The NSE(log) is more suitable than NSE for
435 baseflow evaluation because baseflow is typically a few orders of magnitude smaller
436 than the quick flow that is generated during the heavy rainfall events. The r is the
437 most straightforward criterion among these four selected criteria to indicate the
438 storage-discharge relationship is linear or nonlinear because r measures the degree of

439 linear association between the observed and simulated baseflow. Thus r is the most
 440 powerful criterion to evaluate baseflow generation structure in this study. Moreover,
 441 higher value of linear correlation coefficient (r) is the precondition for higher value of
 442 Nash-Sutcliffe efficiency (NSE) because NSE can be decomposed three components
 443 as advised by [Murphy \(1988\)](#):

$$444 \quad NSE = A - M - N = r^2 - \left[r - \left(\frac{\sigma_s}{\sigma_0} \right) \right]^2 - [(\mu_s - \mu_0)/\sigma_0]^2 \quad (31)$$

445 where r is the linear correlation coefficient; (μ_s, σ_s) and (μ_0, σ_0) represent the
 446 first two statistical moments (means and standard deviations) of simulated and
 447 observed sequences, respectively. The quantity A measures the strength of the linear
 448 relationship between simulated and observed values, M measures the conditional
 449 bias, and N measures the unconditional bias. Higher value of NSE depends on higher
 450 A , as well as lower M and N . That is to say, higher NSE is achieved by both higher
 451 r and lower bias. In this study, the much high r of modified MWBMs for simulating
 452 baseflow provide precondition for higher value of Nash-Sutcliffe efficiency (NSE).

453 **5.2 Different control of the two parameters in the PA11 method on** 454 **baseflow simulation**

455 The capability of PA11 can be evidenced by comparison of the variability and
 456 magnitude of simulated and observed baseflow, which can be measured by r and B ,
 457 respectively. The variability and magnitude of baseflow are controlled by different

458 parameters in the PA11 approach. The PA11 method (equation (23)) can be
459 reformulated as:

$$460 \quad Q_b(t) = e^{\left(\frac{W(t)}{m}\right)} / e^{b/m} \quad (32)$$

461 In terms of equation (32), it can be found that the value of simulated baseflow is
462 determined by two parts. One is the nonlinearity between $W(t)$ and $Q_b(t)$ (*i.e.*
463 $e^{\left(\frac{W(t)}{m}\right)}$). The other is the magnitude of $Q_b(t)$ (*i.e.* $e^{b/m}$). The first part represents
464 the nonlinear structure between $W(t)$ and $Q_b(t)$, and the second part only includes
465 parameters m and b . Linear correlation (r) between observed and simulated baseflow
466 is only controlled by the first part. Thus the criterion r is the most direct criterion to
467 evaluate whether the storage-discharge relationship is exponential nonlinear or not,
468 which is controlled only by m . In other words, the ability of the nonlinear baseflow
469 modelling structure is only controlled by parameter m and directly measured by
470 criterion r . While NSE(log), NSE and B are determined by both parts, which is
471 controlled by both m and b .

472 Take DWBM as an example, DWBM_{mod} (*i.e.* modified DWBM) can capture the
473 variability of baseflow, but it underestimates apparently the magnitude of baseflow.
474 **Figure 9** shows observed and simulated baseflow sequences for catchment 238204
475 (**Figure 9a**) and catchment 108002 (**Figure 9b**). The baseflow simulated by DWBM_{mod}
476 has the same variability with the observed baseflow, *i.e.* both increase and decrease
477 simultaneously and peak at the same time, but they have different magnitudes.

478 The differences in magnitude can be attributed two reasons. One is the
479 uncertainty of observed baseflow derived from the LH method. The other one is the
480 poorly calibrated parameter b , which determines the magnitude of simulated baseflow.
481 The magnitude of baseflow is much smaller than that of total and direct flows, thus
482 the magnitude of baseflow is easily poorly simulated with poorly calibrated parameter
483 b . [Figure 10](#) shows the comparison of baseflow derived from LH method used in this
484 study with the other two baseflow separation methods, *i.e.* the United Kingdom
485 Institute of Hydrology (UKIH) method ([Richards, 1994](#)) and the Chapman-Maxwell
486 (CM) method ([Chapman and Maxwell, 1996](#)). Baseflows derived from three digital
487 filter methods have the same temporal variability, but have different magnitudes.
488 Considering that the variability of baseflow has been well captured by DWBM_{mod}, the
489 difference in magnitude can be further reduced by adjusting the parameter b . In
490 DWBM_{mod}, under-estimation of baseflow in catchment 238204 means b is
491 overestimated. Over-estimation of baseflow in catchment 108002 means b is
492 underestimated. The poorly calibrated b in DWBM_{mod} is adjusted (hereafter denoted
493 as Adjusted- b -DWBM_{mod}) through minimizing the criterion B . The details of
494 adjustment of parameter b is shown in [Table 8](#). As shown in [Figure 11](#), the magnitude
495 difference of baseflow decreased in Adjusted- b -DWBM_{mod} with higher NSE, NSE
496 (log) and B than those of both DWBM and DWBM_{mod}.

497 However, in this study, further calibration of baseflow parameters (b) against the
498 separated (or “observed”) baseflow to get a better performance of baseflow is not

499 considered. The calibration procedure adapted in this study is to calibrate both
500 original and modified models against total flow only due to lack of direct
501 measurement of baseflow. Separated slow component from hydrographs using widely
502 used baseflow separation method (such as the LH method used in this study) may not
503 be strictly considered as baseflow (Klaus and McDonnell, 2013; Pelletier and
504 Andreassian, 2020). The baseflow modelling structure of modified MWBMs can
505 catch the variability of baseflow from all three digital filter methods, the magnitude
506 difference of baseflow can attribute to the uncertainty of baseflow separation method
507 and calibration process. Here we just demonstrate the superiority of nonlinear
508 baseflow modelling structure to capture storage baseflow dynamics at monthly time
509 scale. More studies on the calibration of baseflow parameters still required in the
510 future to improve the performance of MWBMs. But it is beyond the scope of this
511 study.

512 **5.3 Considering model consistency for structure evaluation**

513 The structure of the five models consists of several components, representing
514 different hydrological processes. The evaluation of baseflow performance can be
515 referred as “model consistency” evaluation, defined as the ability of a model structure
516 to adequately reproduce several hydrological signatures simultaneously while using
517 the same set of parameter values (Euser et al., 2013). The consistency is considered
518 important for evaluating model structure because consistency can achieve the realistic
519 representation of the real world and reduce equifinality (McMillan, 2020). The

520 improved performance of baseflow using modified MWBMs is resulted from more
521 reasonable baseflow modelling structure with only one more parameter rather than
522 overparameterization or equifinality and thus overparameterization is not evaluated
523 here.

524 In this study, the general improvement of baseflow performance indicated that
525 the nonlinear storage-baseflow relationship can improve the consistency of MWBMs.
526 The more realistic modelling in the modified MWBMs can achieve the least
527 uncertainty in simulating not only total streamflow (Kumar, 2011) but also baseflow.
528 During past few decades, both quantity and quality of baseflow have received
529 increased attention (Arnold et al., 1995) such as sustaining aquatic habitats (Poff et al.,
530 1997; Fan et al., 2013) and dynamics of chemicals in watersheds (Shafii et al., 2019).
531 Accurate simulation of baseflow in MWBMs will extend the capability and
532 application of MWBMs. Thus, improvement of MWBMs structure should consider
533 the consistency of several hydrological signatures to achieve realism of hydrological
534 processes instead of focusing on total streamflow only.

535 **5.4 Nonlinear exponential storage-discharge relationship for** 536 **baseflow estimation**

537 The rationality and physical basis of the nonlinear exponential storage-discharge
538 relationship to describe baseflow process have been proved by previous studies
539 through reservoir conceptualization and recession analysis (Brutsaert and Nieber,

540 1977; Stoelzle et al., 2015; Nippgen et al., 2016). The baseflow process is complex
541 and nonlinear due to the joint control of hydroclimatic conditions and geological
542 characteristics on baseflow generation (Maneta et al., 2018). The nonlinearity of
543 baseflow process is widely observed in catchment storage-discharge relationship. At
544 lower baseflows, large changes in soil moisture are related to relatively small change
545 in baseflow; while at higher baseflows, small changes in soil moisture result in
546 relatively large changes in baseflow (Nippgen et al., 2016). Based on reservoir
547 conceptualization, the nonlinear storage-discharge relationship is usually described by
548 combination of several linear reservoirs or single nonlinear reservoir (Stoelzle et al.,
549 2015). For the case of single nonlinear reservoir, baseflow is typically estimated using
550 a power function (Harman and Sivapalan, 2009) or an exponential function (Beven
551 and Kirkby, 1979). Peters and Aulenbach (2011) proposed the PA11 model in virtue
552 of observed soil moisture using the exponential function. Aulenbach and Peters (2018)
553 showed that the exponential function (*i.e.* the PA11) can well describe the
554 storage-discharge dynamics with a high coefficient of determination (adjusted
555 $R^2 = 0.96$) using observed soil moisture and estimated baseflow from Eckhardt filter
556 method (Eckhardt, 2005). The nonlinear storage-discharge relationship can also be
557 derived from recession analysis. In Kirchner (2009), the recession curve is described
558 by a power law function $-\frac{dQ}{dt} = aQ^b$ based on the fundamental works of Horton
559 (1941) and Brutsaert and Nieber (1977). The storage-discharge relationship can be
560 linear, power, exponential, or more than exponential when $b=1$, $b<2$, $b=2$, $b>2$,

561 respectively. [Patnaik et al. \(2018\)](#) found the median b of the recession curve of 358
562 catchments in the United States nearly equals to 2, which indicates that the
563 storage-discharge relationship is exponential in most catchments. In this study, the
564 nonlinear exponential storage-discharge relationship in modified MWBMs improve
565 model performance in terms of both total flow and baseflow compared with the linear
566 storage-discharge relationship in original MWBMs. Therefore, the nonlinear
567 exponential storage-discharge relationship may have stronger physical basis and is
568 more universal than linear storage-discharge relationship.

569 **5.5 Monthly versus daily models for baseflow simulation**

570 Within-month variability of hydrological variables and the storage response to
571 daily rainfall events are two main factors that lead to different baseflow generation
572 mechanisms at daily and monthly time scale. The two factors need to be considered in
573 the nonlinear or linear forms of the storage-discharge relationship. Baseflow is
574 possible to be measured at daily and/or hourly time scale through rigorous flow
575 recession analysis ([Cheng et al., 2016](#)) and tracer-based methods ([Gonzales et al.,
576 2009](#)), while it is difficult to be measured at monthly time scale. Based on observed
577 hydrological variables, the nonlinear and linear storage-discharge relationships can be
578 derived from reservoir conceptualization and recession analysis at short time scale (*i.e.*
579 daily and hourly). For the MWBMs investigated in this study, catchment
580 storage-discharge relationships at short time scale are directly adopted without
581 considering the within-month variability in climate forcing variables and

582 rainfall-storage responses. From modelling perspective, Wang et al. (2011) compared
583 the monthly and daily models for the simulation of monthly total runoff and reported
584 that the monthly models have not been disadvantaged for not using the within-month
585 temporal sequences of the forcing variables. The other factor, *i.e.* storage response to
586 rainfall events at current month, is ignored by original MWBMs and leads to apparent
587 lag in the peak time of baseflow. Increased correlation between observed and
588 simulated baseflow using modified MWBMs is probably resulted from adding
589 precipitation to storage at the current month for baseflow generation. No obvious
590 correlation has been found between increased model performance on baseflow and
591 catchments properties such as aridity index, elevation, slope, soil properties, *etc.* From
592 a modelling perspective, monthly storage-baseflow relationship is investigated in this
593 study and results indicate that the nonlinear relationship is more effectively to capture
594 the variability of monthly baseflow at most catchments. However, further studies are
595 still required to advance our capability in simulating baseflow across various spatial
596 and time scales.

597 **6 Conclusions**

598 In this study, the performance of linear storage-discharge relationship in 5 widely
599 used monthly water balance models (MWBMs) is diagnosed and evaluated using
600 observed daily hydrological data from 443 catchments across Australia with distinct
601 hydro-climatic conditions. A nonlinear exponential storage-discharge relationship (*i.e.*
602 the PA11) is employed to replace the linear one in the study MWBMs to improve

603 monthly baseflow modelling accuracy and to achieve realistic hydrological modelling
604 at monthly time scale. The main findings are summarized as follows:

605 (1). Baseflow simulated by 5 original MWBMs are remarkably underestimated
606 and unable to explain the dispersion of observed baseflow. The poor performance of
607 baseflow suggests the linear baseflow generation mechanism may not be suitable for
608 monthly water balance models.

609 (2). Modified MWBMs with nonlinear baseflow modelling structure outperform
610 the original ones in simulating total flow. On average, the criteria NSE, NSE(log), r
611 and B of modified models are improved in $82\pm 4.0\%$, $72\pm 4.7\%$, $76\%\pm 4.5\%$ and
612 $51\pm 2.4\%$ of study catchments, respectively.

613 (3). The modified MWBMs improve baseflow performance significantly with
614 better NSE(log) and r in $68\pm 4.6\%$ and $83\pm 4.1\%$ study catchments with median
615 improvement of 0.17 ± 0.03 and 0.14 ± 0.03 , respectively.

616 These results suggest that the modified MWBMs with the nonlinear
617 storage-discharge relationship is more capable than the original MWBMs with the
618 linear storage-discharge relationship to capture the dynamics in monthly baseflow
619 component.

620

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629

630 Appendix A. Model description

631 A.1. Dynamic Water Balance Model (DWBM)

632 The DWBM was proposed by Zhang et al. (2008) based on the Budyko
633 framework. The model structure is presented in Figure 2a. The DWBM
634 conceptualizes a catchment as a system of two storages, *i.e.* soil water storage and
635 groundwater storage. Rainfall in time step t is partitioned into quick flow (Q_d) and the
636 sum of the other water balance components. The $Q_d(t)$ in the DWBM is calculated as:

$$637 \quad Q_d(t) = P(t) - X(t) \quad (4)$$

638 where $X(t)$ is called catchment rainfall retention, calculated as

639 $X(t) = P(t)F\left(\frac{PET(t)+S_{max}-S(t-1)}{P(t)}, a_1\right)$. The parameter S_{max} is soil water storage

640 capacity. a_1 is a parameter that influences retention efficiency. The form of

641 $F\left(\frac{PET(t)+S_{max}-S(t-1)}{P(t)}, a_1\right)$ is generalized from the equation $1 + \frac{PET(t)+S_{max}-S(t-1)}{P(t)} -$

642 $[1 + (\frac{PET(t)+S_{max}-S(t-1)}{P(t)})^{a_1}]^{1/a_1}$, which is a classical Budyko framework proposed by

643 (Fu, 1981). The form of $F()$ used following is the same. Water availability of a

644 catchment can be defined as $W(t) = X(t) + S(t - 1)$. The $W(t)$ is the amount of

645 rainfall retained in the catchment for actual evapotranspiration, soil moisture and

646 groundwater recharge. Namely, $W(t) = E_a(t) + S(t) + R(t)$. The $E_a(t)$ is

647 estimated as:

$$648 \quad E_a(t) = W(t) \times F\left(\frac{PET(t)}{W(t)}, a_2\right) \quad (5)$$

649 where a_2 is a parameter that influences evapotranspiration efficiency. Groundwater
 650 recharge (R) is also generated from $W(t)$ and is calculated as:

$$651 \quad R(t) = W(t) - Y(t) \quad (6)$$

652 where $Y(t)$ is called evapotranspiration opportunity, calculated as
 653 $Y(t) = W(t)F\left(\frac{PET(t)+S_{max}}{W(t)}, a_2\right)$. Groundwater discharge in the DWBM is treated as
 654 linear reservoir and $Q_b(t)$ is calculated as:

$$655 \quad Q_b(t) = dG(t - 1) \quad (7)$$

656 where the parameter d represents the baseflow generation efficiency. Groundwater
 657 balance can be modeled as $G(t) = (1 - d)G(t - 1) + R(t)$. In total, there are 4
 658 parameters in the DWBM to be calibrated including S_{max} , a_1 , a_2 and d . The unit of
 659 S_{max} is mm and the unit of d is month⁻¹.

660 **A.2. Belgium Model (VUB)**

661 The VUB was proposed by Vandewiele et al. (1992). The model structure is
 662 presented in Figure 2b. In this model, actual evapotranspiration (E_a) is computed as:

$$663 \quad E_a(t) = \min \left[PET(t) \times \left(1 - x_1 \frac{W(t)}{PET(t)} \right), W(t) \right] \quad (8)$$

664 where the x_1 is a non-negative parameter which represents evapotranspiration
 665 resistance of the river basin; $W(t)$ is available water for E_a and is estimated as
 666 $W(t) = P(t) + S(t - 1)$. Simulated monthly total flow of the VUB is the sum of

667 quick flow (Q_d) and baseflow (Q_b). The $Q_d(t)$ is calculated as a function of soil
668 moisture and effective precipitation as:

$$669 \quad Q_d(t) = x_3 S(t-1) \times P_e(t) \quad (9)$$

$$670 \quad P_e(t) = P(t) - PET(t) \times \left(1 - e^{\frac{-P(t)}{PET(t)}}\right) \quad (10)$$

671 where x_3 is a parameter, representing the fraction of precipitation that is immediately
672 transformed into Q_d during the same rainfall event; $P_e(t)$ is the effective
673 precipitation. The Q_b in month t is calculated as:

$$674 \quad Q_b(t) = x_2 S(t-1) \quad (11)$$

675 where x_2 is a parameter, representing the fraction of stored soil water that is
676 discharged as baseflow. In total, 3 parameters in the VUB are to be calibrated
677 including x_1 , x_2 and x_3 . The unit of x_2 and x_3 is month^{-1} .

678 **A.3. Time Variant Gain Model (TVGM)**

679 The theory of the TVGM was first proposed by [Xia et al. \(1997\)](#) and then
680 developed later by [Xia et al. \(2005\)](#). The model structure is presented in [Figure 2c](#). As
681 for the actual evapotranspiration (E_a), it can be expressed as a function of soil
682 moisture and potential evapotranspiration as:

$$683 \quad E_a(t) = PET(t) \times (S(t-1)/S_{max})^\gamma \quad (12)$$

684 where γ is a parameter, representing the nonlinear relationship between E_a and
685 relative soil moisture. Quick flow (Q_d) in month t is calculated as:

686
$$Q_d(t) = g_1(S(t-1)/S_{max})^{g_2} \times P(t) \quad (13)$$

687 where S_{max} is saturated soil moisture; g_1 and g_2 are two empirical coefficients. As
 688 for the subsurface runoff generation model, the soil moisture at time step t is
 689 calculated by combining the water balance equation and the dynamic
 690 storage-discharge function. And the baseflow (Q_b) is calculated by a linear function of
 691 the soil moisture at time steps $t-1$ and t :

692
$$Q_b(t) = k_r (S(t-1) + S(t))/2 \quad (14)$$

693 where $S(t-1)$ and $S(t)$ are the soil moisture at time $t-1$ and t , respectively; k_r is an
 694 empirical coefficient related to baseflow generation. In total, there are 5 parameters in
 695 the TVGM to be calibrated including S_{max} , γ , g_1 , g_2 and k_r . The unit of S_{max} is
 696 mm and the unit of k_r is month^{-1} .

697 **A.4. WatBal Model (WM)**

698 The WM was originally developed by [Leaf and Brink \(1973\)](#) and was further
 699 modified by [Wang et al. \(2014\)](#). The model structure is presented in [Figure 2d](#). $E_a(t)$
 700 is a function of potential evapotranspiration and the relative soil moisture and is
 701 estimated as:

702
$$E_a(t) = PET(t) \times S(t-1)/S_{max} \quad (15)$$

703 where $S(t-1)$ is the soil moisture storage at the beginning of time step t ; S_{max} is
 704 the maximum storage capacity. $Q_d(t)$ is calculated as a function of relative storage
 705 and precipitation as:

706
$$Q_d(t) = k_s P(t) \times S(t - 1) / S_{max} \quad (16)$$

707 where k_s is the quick flow coefficient. Q_b in month t is calculated with a linear
 708 storage-discharge function as:

709
$$Q_b(t) = k_g S(t - 1) \quad (17)$$

710 where k_g is a parameter, representing the fraction of stored soil water that discharges
 711 as baseflow. In total, there are 3 parameters in the WM to be calibrated including
 712 S_{max} , k_s and k_g . The unit of S_{max} is mm and the unit of k_g is month⁻¹.

713 **A.5. Schaake Model (SM)**

714 The SM was firstly developed by [Schaake and Liu \(1989\)](#) and was improved later
 715 by [Schaake \(1990\)](#). The model structure is presented in [Figure 2e](#). The uniqueness of
 716 the model is to introduce soil moisture deficit (D) for estimation of actual
 717 evapotranspiration (E_a) and runoff. In the SM, E_a is assumed to deplete the soil
 718 water at a potential rate when the storage deficit is zero, whereas E_a is zero when the
 719 storage deficit reaches the maximum. For the case storage deficit does not reach the
 720 maximum, E_a of month t is calculated as:

721
$$E_a(t) = PET(t) \times \frac{D_{max} - D(t)}{D_{max}} \quad (18)$$

722 where $D(t)$ is the soil water storage deficit at current time step, and D_{max} is the
 723 maximum deficit of soil moisture storage. Quick runoff (Q_d) is calculated as:

724
$$Q_d(t) = P_e(t)^2 / (P_e(t) + D_{max}) \quad (19)$$

725
$$P_e(t) = P(t) - \theta E_a(t) - zD(t) \quad (20)$$

726 where $P_e(t)$ is effective precipitation; and θ and z are empirical parameters.
 727 Parameter θ represents the proportion of actual evapotranspiration that must be
 728 satisfied by current month precipitation before runoff can occur, and parameter z
 729 represents the proportion of infiltration that must be satisfied by current month
 730 precipitation before runoff can occur. Baseflow (Q_b) is assumed to vary with soil
 731 moisture deficit (*i.e.* D) and is calculated as:

732
$$Q_b(t) = k(G_{max} - D(t)) \quad (21)$$

733 where k is a parameter representing the proportion of surplus to generate baseflow,
 734 and G_{max} is the maximum groundwater storage. In total, there are 5 parameters in
 735 the SM to be calibrated including D_{max} , θ , z , k and G_{max} . The unit of D_{max} and
 736 G_{max} is mm and the unit of k is month⁻¹. The storage structure of SM is different
 737 with the other four. The SM uses only one soil moisture deficit (D) to represent both
 738 soil water and groundwater storages and uses two parameters (*i.e.* D_{max} and G_{max}) to
 739 represent the capacity of soil water and groundwater storages, respectively. Recharge
 740 from soil moisture to groundwater is not allowed in the SM. $Q_d(t)$ is calculated as
 741 the function of $-D(t)$, $E_a(t)$ is a function of $(D_{max}-D(t))$, and $Q_b(t)$ is a function of
 742 $(G_{max}-D(t))$. The number of water storage is regarded as only one (Jiang et al., 2007)
 743 as there is only one current status of moisture storage (*i.e.* D).

744

745 **References**

746

747 Ahiablame, L., Chaubey, I., Engel, B., Cherkauer, K. and Merwade, V., 2013. Estimation of
748 annual baseflow at ungauged sites in indiana usa. *J. Hydrol*, 476: 13-27.

749 Aksoy, H. and Wittenberg, H., 2011. Nonlinear baseflow recession analysis in watersheds
750 with intermittent streamflow. *Hydrological Sciences Journal*, 56(2): 226-237.

751 Alley, W.M., 1984. On the treatment of evapotranspiration, soil moisture accounting, and
752 aquifer recharge in monthly water balance models. *Water Resour Res*, 20(8):
753 1137-1149.

754 Arnold, J.G., Allen, P.M., Muttiah, R. and Bernhardt, G., 1995. Automated base flow
755 separation and recession analysis techniques. *Groundwater*, 33(6): 1010-1018.

756 Aulenbach, B.T. and Peters, N.E., 2018. Quantifying climate-related interactions in shallow
757 and deep storage and evapotranspiration in a forested, seasonally water-limited
758 watershed in the southeastern united states. *Water Resour Res*, 54(4): 3037-3061.

759 Bai, P., Liu, X. and Liu, C., 2018. Improving hydrological simulations by incorporating grace
760 data for model calibration. *J. Hydrol*, 557: 291-304.

761 Bai, P., Liu, X., Liang, K. and Liu, C., 2015. Comparison of performance of twelve monthly
762 water balance models in different climatic catchments of china. *J. Hydrol*, 529:
763 1030-1040.

764 Bastola, S., Murphy, C. and Sweeney, J., 2011. The role of hydrological modelling
765 uncertainties in climate change impact assessments of irish river catchments. *Adv Water
766 Resour*, 34(5): 562-576.

767 Beven, K., 1993. Prophecy, reality and uncertainty in distributed hydrological modelling. *Adv
768 Water Resour*, 16(1): 41-51.

769 Beven, K.J., 2001. rainfall-runoff modelling - the primer. Wiley, Chichester, UK.

770 Beven, K.J. and Kirkby, M.J., 1979. A physically based, variable contributing area model of
771 basin hydrology. *Hydrological Sciences Bulletin*, 24(1): 43-69.

772 Brutsaert, W. and Nieber, J.L., 1977. Regionalized drought flow hydrographs from a mature
773 glacial plateau. *Water Resour Res*, 13(3): 637-643.

774 Buttle, J.M., 1994. Isotope hydrograph separations and rapid delivery of pre-event from
775 drainage basins. *Prog Phys Geog*, 18(1): 16-41.

776 Chapman, T., 1999. A comparison of algorithms for stream flow recession and baseflow
777 separation. *Hydrol Process*, 13(5): 701-714.

778 Chen, J., Brissette, F.P., Poulin, A. and Leconte, R., 2011. Overall uncertainty study of the
779 hydrological impacts of climate change for a canadian watershed. *Water Resour Res*, 47:
780 W12509.

781 Cheng, L., Yaeger, M., Viglione, A., Coopersmith, E., Ye, S. and Sivapalan, M., 2012. Exploring
782 the physical controls of regional patterns of flow duration curves - part 1: insights from
783 statistical analyses. *Hydrol Earth Syst Sc*, 16(11): 4435-4446.

784 Cheng, L., Zhang, L. and Brutsaert, W., 2016. Automated selection of pure base flows from
785 regular daily streamflow data: objective algorithm. *J. Hydrol Eng*, 21(11): 06016008.

786 Dakhlaoui, H., Ruelland, D., Trambly, Y. and Bargaoui, Z., 2017. Evaluating the robustness of
787 conceptual rainfall-runoff models under climate variability in northern tunisia. *J. Hydrol*,
788 550: 201-217.

789 Deng, C., Liu, P., Wang, D. and Wang, W., 2018. Temporal variation and scaling of
790 parameters for a monthly hydrologic model. *J. Hydrol*, 558: 290-300.

791 Duan, Q., Schaake, J., Andréassian, V., Franks, S., Goteti, G. and Gupta, H.V. et al., 2006.
792 Model parameter estimation experiment (mopex): an overview of science strategy and
793 major results from the second and third workshops. *J. Hydrol*, 320(1): 3-17.

794 Duffy, C.J., 1996. A two-state integral-balance model for soil moisture and groundwater
795 dynamics in complex terrain. *Water Resour Res*, 32(8): 2421-2434.

796 Eckhardt, K., 2005. How to construct recursive digital filters for baseflow separation. *Hydrol*
797 *Process*, 19(2): 507-515.

798 Euser, T., Winsemius, H.C., Hrachowitz, M., Fenicia, F., Uhlenbrook, S. and Savenije, H.H.G.,
799 2013. A framework to assess the realism of model structures using hydrological
800 signatures. *Hydrol. Earth Syst. Sci.*, 17(5): 1893-1912.

801 Fan, Y., Li, H. and Miguez-Macho, G., 2013. Global patterns of groundwater table depth.
802 *Science*, 339(6122): 940.

803 Fenicia, F., Savenije, H.H.G., Matgen, P., Pfister, L. and Abebe, A., 2006. Is the groundwater
804 reservoir linear? Learning from data in hydrological modelling. *Hydrol. Earth Syst. Sci.*,
805 10(1): 139-150.

806 Fowler, K., Coxon, G., Freer, J., Peel, M., Wagener, T. and Western, A. et al., 2018b.
807 Simulating runoff under changing climatic conditions: a framework for model
808 improvement. *Water Resour Res*, 54(12): 9812-9832.

809 Fowler, K., Peel, M., Western, A. and Zhang, L., 2018a. Improved rainfall-runoff calibration
810 for drying climate: choice of objective function. *Water Resour Res*, 54(5): 3392-3408.

811 Fu, 1981. On the calculation of the evaporation from land surface. *SCIENTIA ATMOSPHERICA*
812 *SINICA*, 5(1): 23-31.

813 Gleick, P.H., 1987. The development and testing of a water balance model for climate impact
814 assessment: modeling the sacramento basin. *Water Resour Res*, 23(6): 1049-1061.

815 Gonzales, A.L., Nonner, J., Heijkers, J. and Uhlenbrook, S., 2009. Comparison of different
816 base flow separation methods in a lowland catchment. *Hydrol. Earth Syst. Sci.*, 13:
817 2055-2068.

818 Grefenstette, J.J., Member and leee, 1986. Optimization of control parameters for genetic
819 algorithms. *IEEE Transactions on Systems, Man, and Cybernetics*, 16(1): 122-128.

820 Gupta, H.V., Kling, H., Yilmaz, K.K. and Martinez, G.F., 2009. Decomposition of the mean
821 squared error and nse performance criteria: implications for improving hydrological
822 modelling. *J. Hydrol*, 377(1): 80-91.

823 Gupta, H.V., Wagener, T. and Liu, Y., 2008. Reconciling theory with observations: elements of
824 a diagnostic approach to model evaluation. *Hydrol Process*, 22(18): 3802-3813.

825 Hamel, P., Guswa, A.J., Sahl, J., Zhang, L. and Abebe, A., 2017. Predicting dry-season flows
826 with a monthly rainfall-runoff model: performance for gauged and ungauged
827 catchments. *Hydrol Process*, 31(22): 3844-3858.

828 Harman, C. and Sivapalan, M., 2009. A similarity framework to assess controls on shallow
829 subsurface flow dynamics in hillslopes. *Water Resour Res*, 45: W01417.

830 He, Z., Vorogushyn, S., Unger-Shayesteh, K., Gafurov, A., Kalashnikova, O. and Omorova, E. et
831 al., 2018. The value of hydrograph partitioning curves for calibrating hydrological
832 models in glacierized basins. *Water Resour Res*, 54(3): 2336-2361.

833 Horton, R.E., 1941. Virtual channel-inflow graphs. *Eos Transactions American Geophysical*
834 *Union*, 22(3): 811-820.

835 Hrachowitz, M., Fovet, O., Ruiz, L., Euser, T., Gharari, S. and Nijzink, R. et al., 2014. Process
836 consistency in models: the importance of system signatures, expert knowledge, and
837 process complexity. *Water Resour Res*, 50(9): 7445-7469.

838 Jiang, T., Chen, Y.D., Xu, C., Chen, X., Chen, X. and Singh, V.P., 2007. Comparison of
839 hydrological impacts of climate change simulated by six hydrological models in the
840 dongjiang basin, south china. *J. Hydrol*, 336(3-4): 316-333.

841 Kelleher, C., McGlynn, B. and Wagener, T., 2017. Characterizing and reducing equifinality by
842 constraining a distributed catchment model with regional signatures, local observations,
843 and process understanding. *Hydrol Earth Syst Sc*, 21(7): 3325-3352.

844 Khatami, S., Peel, M.C., Peterson, T.J. and Western, A.W., 2019. Equifinality and flux mapping:
845 a new approach to model evaluation and process representation under uncertainty.
846 *Water Resour Res*, 55(11): 8922-8941.

847 Kirchner, J.W., 2009. Catchments as simple dynamical systems: catchment characterization,
848 rainfall-runoff modeling, and doing hydrology backward. *Water Resour Res*, 45:
849 W02429.

850 Kottek, M., Grieser, J., Beck, C., Rudolf, B. and Rubel, F., 2006. World map of the
851 köppen-geiger climate classification updated. *Meteorol Z.*, 15(3): 259-263.

852 Krause, P., Boyle, D.P. and Bäse, F., 2005. Comparison of different efficiency criteria for
853 hydrological model assessment. *Advances in Geosciences*, 5: 89-97.

854 Kumar, P., 2011. Typology of hydrologic predictability. *Water Resour Res*, 47: W00H05.

855 Larabi, S., St-Hilaire, A., Chebana, F. and Latraverse, M., 2018. Multi-criteria process-based
856 calibration using functional data analysis to improve hydrological model realism. *Water*
857 *Resour Manag*, 32(1): 195-211.

858 Leaf, C. and Brink, G., 1973. Computer simulation of snowmelt with a colorado subalpine
859 watershed. *For. Serv. Res. Pap.*, RM-99.

860 Lee, D., 2007. Testing a conceptual hillslope recession model based on the storage-discharge
861 relationship with the richards equation. *Hydrol Process*, 21(23): 3155-3161.

862 Legates, D.R. and McCabe Jr., G.J., 1999. Evaluating the use of "goodness-of-fit" measures in
863 hydrologic and hydroclimatic model validation. *Water Resour Res*, 35(1): 233-241.

864 Lindstrom, G., Johansson, B., Persson, M., Gardelin, M. and Bergstrom, S., 1997.
865 Development and test of the distributed hbv-96 hydrological model. *J. Hydrol*, 201(1-4):
866 272-288.

867 Lyne, V. and Hollick, M., 1979. Stochastic time-variable rainfall-runoff modelling,
868 Proceedings of the Hydrology and Water Resources Symposium. Institute of Engineers
869 Australia National Conference, Perth.

870 Maneta, M.P., Soulsby, C., Kuppel, S. and Tetzlaff, D., 2018. Conceptualizing catchment
871 storage dynamics and nonlinearities. *Hydrol Process*, 32: 3299-3303.

872 Markovic, D. and Koch, M., 2015. Stream response to precipitation variability: a spectral
873 view based on analysis and modelling of hydrological cycle components. *Hydrol Process*,
874 29(7): 1806-1816.

875 McMillan, H., 2020. Linking hydrologic signatures to hydrologic processes: a review. *Hydrol*
876 *Process*, 34: 1393-1409.

877 Moore, R.D., 1997. Storage-outflow modelling of streamflow recessions, with application to
878 a shallow-soil forested catchment. *J. Hydrol*, 198(1): 260-270.

879 Murphy, A.H., 1988. Skill scores based on the mean square error and their relationships to
880 the correlation coefficient. *Mon Weather Rev*, 116(12): 2417-2424.

881 Nash, J.E. and Sutcliffe, J.V., 1970. River flow forecasting through conceptual models, part i -
882 a discussion of principles. *J. Hydrol*, 10: 282-290.

883 Nasser, M., Zahraie, B., Ajami, N.K. and Solomatine, D.P., 2014. Monthly water balance
884 modeling: probabilistic, possibilistic and hybrid methods for model combination and
885 ensemble simulation. *J. Hydrol*, 511: 675-691.

886 Nathan, R.J. and McMahon, T.A., 1990. Evaluation of automated techniques for base flow
887 and recession analyses. *Water Resour Res*, 26(7): 1465-1473.

888 Nippgen, F., McGlynn, B.L., Emanuel, R.E. and Vose, J.M., 2016. Watershed memory at the
889 coveeta hydrologic laboratory: the effect of past precipitation and storage on
890 hydrologic response. *Water Resour Res*, 52(3): 1673-1695.

891 Patnaik, S., Biswal, B., Nagesh Kumar, D. and Sivakumar, B., 2018. Regional variation of
892 recession flow power-law exponent. *Hydrol Process*, 32(7): 866-872.

893 Peters, N.E. and Aulenbach, B.T., 2011. Water storage at the panola mountain research
894 watershed, georgia, usa. *Hydrol Process*, 25: 3878-3889.

895 Poff, L.R., Allan, J.D., Bain, M.B., Karr, J.R., Prestegard, K.L. and Richter, B.D. et al., 1997. The
896 natural flow regime. *Bioscience*, 47(11): 769-784.

897 Rezaeianzadeh, M., Stein, A., Tabari, H., Abghari, H., Jalalkamali, N. and Hosseinipour, E.Z. et
898 al., 2013. Assessment of a conceptual hydrological model and artificial neural networks
899 for daily outflows forecasting. *Int. J. Environ. Sci. Te.*, 10(6): 1181-1192.

- 900 Schaake, J., 1990. From climate to flow. In: waggoner, p.e. (Ed.), Cimate change and us water
901 resourses, New York, 177-206 pp.
- 902 Schaake, J.C. and Liu, L.Z., 1989. Development and application of simple water balance
903 models to understand the relationship between climate and water resources. In: kavvas,
904 m.l. (Ed.), New directions for surface water modelling (proceedings of the baltimore
905 symposium, may 1989), 181, 345-352 pp.
- 906 Schar, C., Vasilina, L., Pertziger, F. and Dirren, S., 2004. Seasonal runoff forecasting using
907 precipitation from meteorological data assimilation systems. *J. Hydrometeorol*, 5(5):
908 959-973.
- 909 Schuite, J., Flipo, N., Massei, N., Rivière, A. and Baratelli, F., 2019. Improving the spectral
910 analysis of hydrological signals to efficiently constrain watershed properties. *Water
911 Resour Res*, 55: 4043-4065.
- 912 Shafii, M. and Tolson, B., 2015. Optimizing hydrological consistency by incorporating
913 hydrological signatures into model calibration objectives. *Water Resour Res*, 51:
914 3796-3814.
- 915 Shafii, M., Basu, N., Craig, J., L. Schiff, S. and Van Cappellen, P., 2017. A diagnostic approach
916 to constraining flow partitioning in hydrologic models using a multiobjective
917 optimization framework. *Water Resour Res*, 53: 3279-3301.
- 918 Shafii, M., Craig, J.R., Macrae, M.L., English, M.C., Schiff, S.L. and Van Cappellen, P. et al.,
919 2019. Can improved flow partitioning in hydrologic models increase biogeochemical
920 predictability? *Water Resour Res*, 55(4): 2939-2960.
- 921 Stoelzle, M., Weiler, M., Stahl, K., Morhard, A. and Schuetz, T., 2015. Is there a superior
922 conceptual groundwater model structure for baseflow simulation? *Hydrol Process*, 29(6):
923 1301-1313.
- 924 Vandewiele, G.L., Xu, C. and Ni-Lar-Win, 1992. Methodology and comparative study of
925 monthly water balance models in belgium, china and burma. *J. Hydrol*, 134(1): 315-347.
- 926 Wang, G.Q., Zhang, J.Y., Jin, J.L., Liu, Y.L., He, R.M. and Bao, Z.X. et al., 2014. Regional
927 calibration of a water balance model for estimating stream flow in ungauged areas of
928 the yellow river basin. *Quatern Int*, 336: 65-72.
- 929 Wang, Q.J., Pagano, T.C., Zhou, S.L., Hapuarachchi, H.A.P., Zhang, L. and Robertson, D.E.,
930 2011. Monthly versus daily water balance models in simulating monthly runoff. *J. Hydrol*,
931 404(3-4): 166-175.
- 932 Westra, S., Thyer, M., Leonard, M., Kavetski, D. and Lambert, M., 2014. A strategy for
933 diagnosing and interpreting hydrological model nonstationarity. *Water Resour Res*, 50:
934 5090-5113.
- 935 Wittenberg, H., 1999. Baseflow recession and recharge as nonlinear storage processes, 13,
936 715-726 pp.
- 937 Xia, J., Connor, K.M.O. and Kachroo, R.K., 1997. A non-linear perturbation model considering
938 catchment wetness and its application in fiver flow forecasting. *J. Hydrol*, 200(1-4):

939 164-178.

940 Xia, J., Wang, G., Tan, G., Ye, A. and Huang, G.H., 2005. Development of distributed
941 time-variant gain model for nonlinear hydrological systems. *Science in China Series D:*
942 *Earth Sciences*, 48(6): 713-723.

943 Xiong, M., Liu, P., Cheng, L., Deng, C., Gui, Z. and Zhang, X. et al., 2019. Identifying
944 time-varying hydrological model parameters to improve simulation efficiency by the
945 ensemble kalman filter: a joint assimilation of streamflow and actual evapotranspiration.
946 *J. Hydrol*, 568: 758-768.

947 Xu, C.Y. and Singh, V.P., 1998. A review on monthly water balance models for water
948 resources investigations. *Water Resour Manag*, 12(1): 20-50.

949 Xu, C.Y., Seibert, J. and Halldin, S., 1996. Regional water balance modelling in the norex area:
950 development and application of monthly water balance models. *J. Hydrol*, 180(1):
951 211-236.

952 Yilmaz, K.K., Gupta, H.V. and Wagener, T., 2008. A process-based diagnostic approach to
953 model evaluation: application to the nws distributed hydrologic model. *Water Resour*
954 *Res*, 44: W09417.

955 Zhang, J., Zhang, Y., Song, J. and Cheng, L., 2017. Evaluating relative merits of four baseflow
956 separation methods in eastern australia. *J. Hydrol*, 549: 252-263.

957 Zhang, L., Potter, N., Hickel, K., Zhang, Y. and Shao, Q., 2008. Water balance modeling over
958 variable time scales based on the budyko framework - model development and testing. *J.*
959 *Hydrol*, 360(1-4): 117-131.

960 Zhang, Y., Viney, N., Frost, A., Oke, A., Brooks, M. and Chen, Y. et al., 2013. Collation of
961 australian modeller's streamflow dataset for 780 unregulated australian catchments,
962 water for a healthy country national research flagship, CSIRO, Australia.

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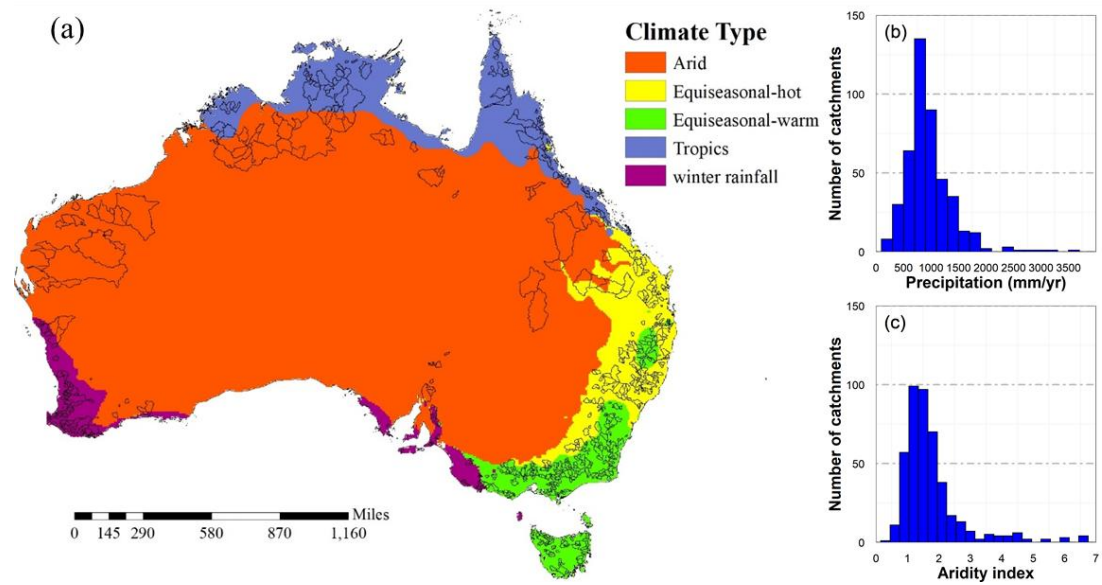


Figure 1. Spatial distribution and catchment characteristics of the 443 unregulated catchments used in this study. The background colour of subplot (a) shows different climatic types based on the Köppen-Geiger classification schemes. Subplots (b) and (c) show the frequency histograms of mean annual precipitation and aridity index, respectively.

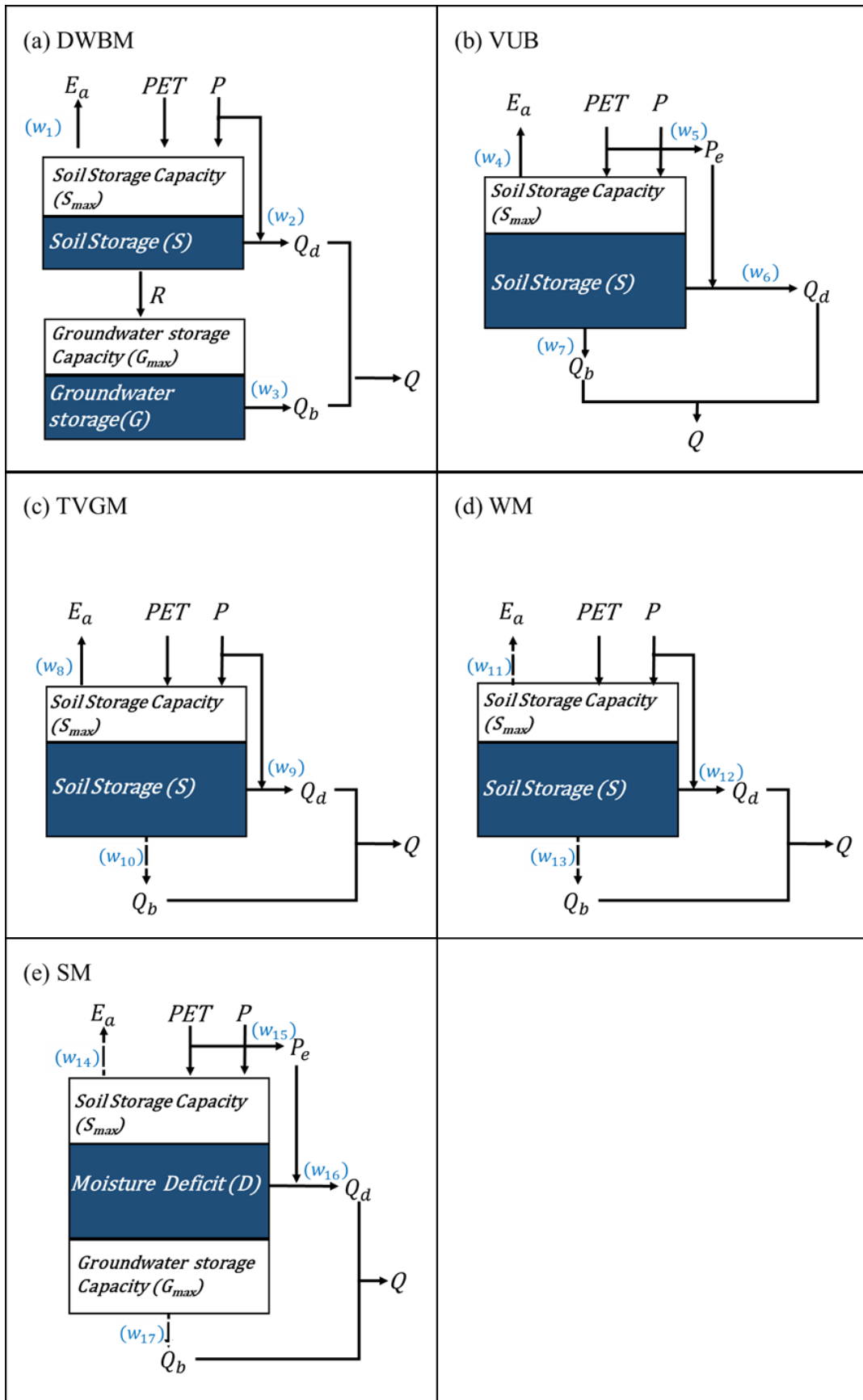


Figure 2. Conceptual representations of the 5 MWBMS with runoff partitioning structure. The meaning of the symbols refers to Table 2.

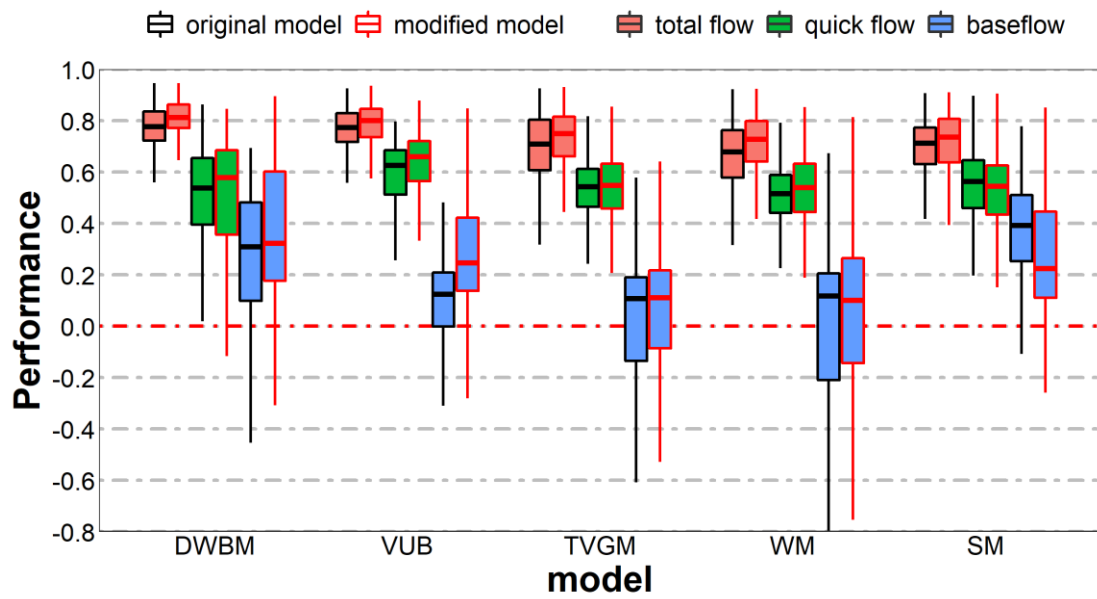


Figure 3. Boxplots showing the performance (value of F_{avg}) of the 5 MWBMs in their original (black line) and modified (red line) forms for estimating total flow (Q , red fill), quick flow (Q_d , green fill) and baseflow (Q_b , blue fill) in all the 443 catchments. Note that the minimum performance of the WM model is not included for a better visualization.

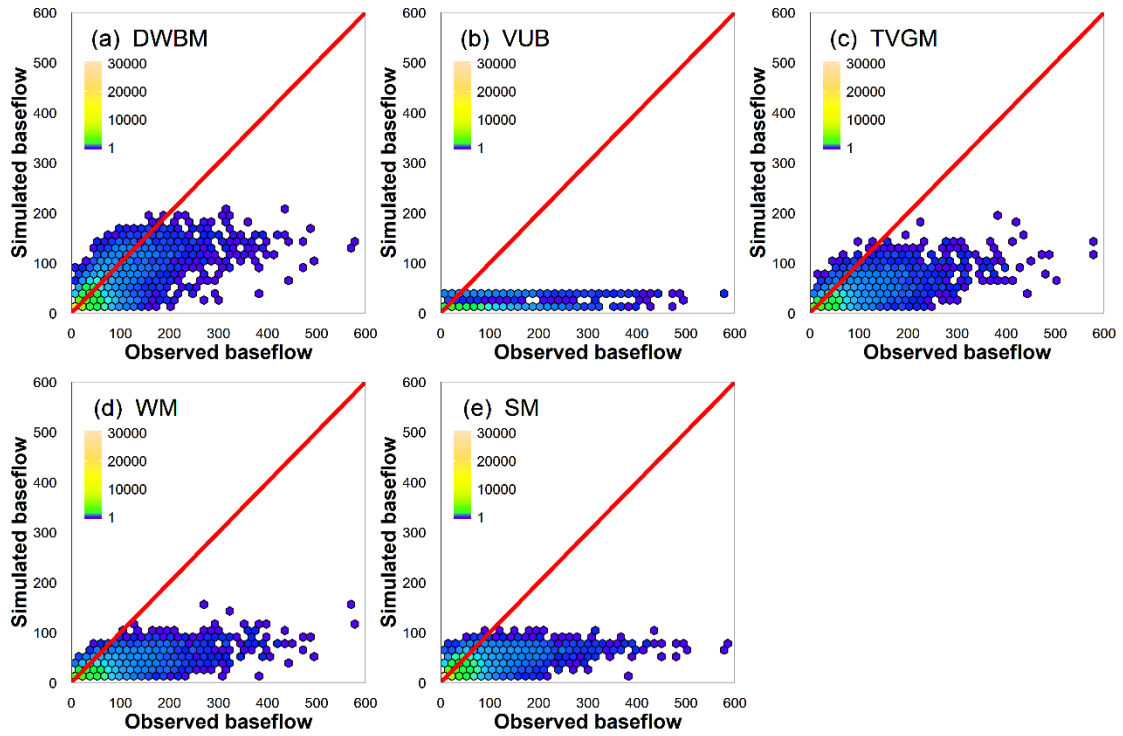


Figure 4. Hexagon binning plots showing comparison of observed and simulated monthly baseflow (mm month⁻¹) by 5 models in their original forms across all the 443 catchments over the period of 1975-2012. Subplots (a)~(e) are the results of DWBM, VUB, TVGM, WM and SM, respectively. The colour ramp of the hexagon in proportion to the counts indicates the density of data points.

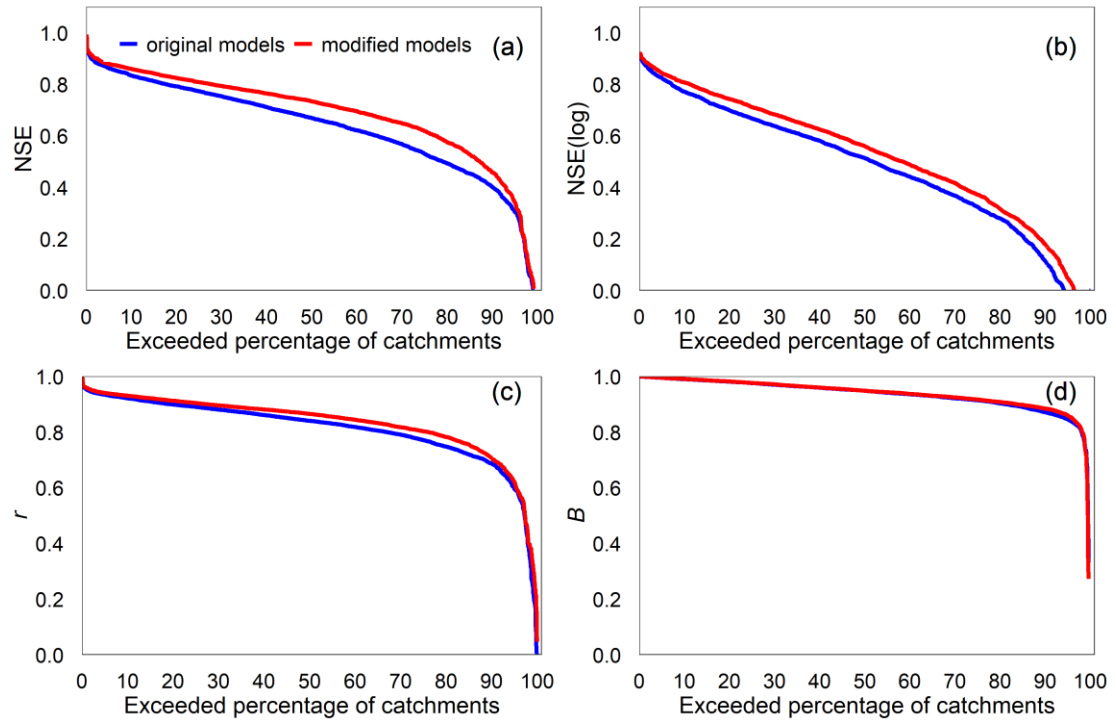


Figure 5. Comparison of total flow performance of the 5 original and modified MWBMs of all the 443 catchments. Subplots show exceeded percentage of catchments that (a) NSE, (b) NSE(log), (c) r , and (d) B .

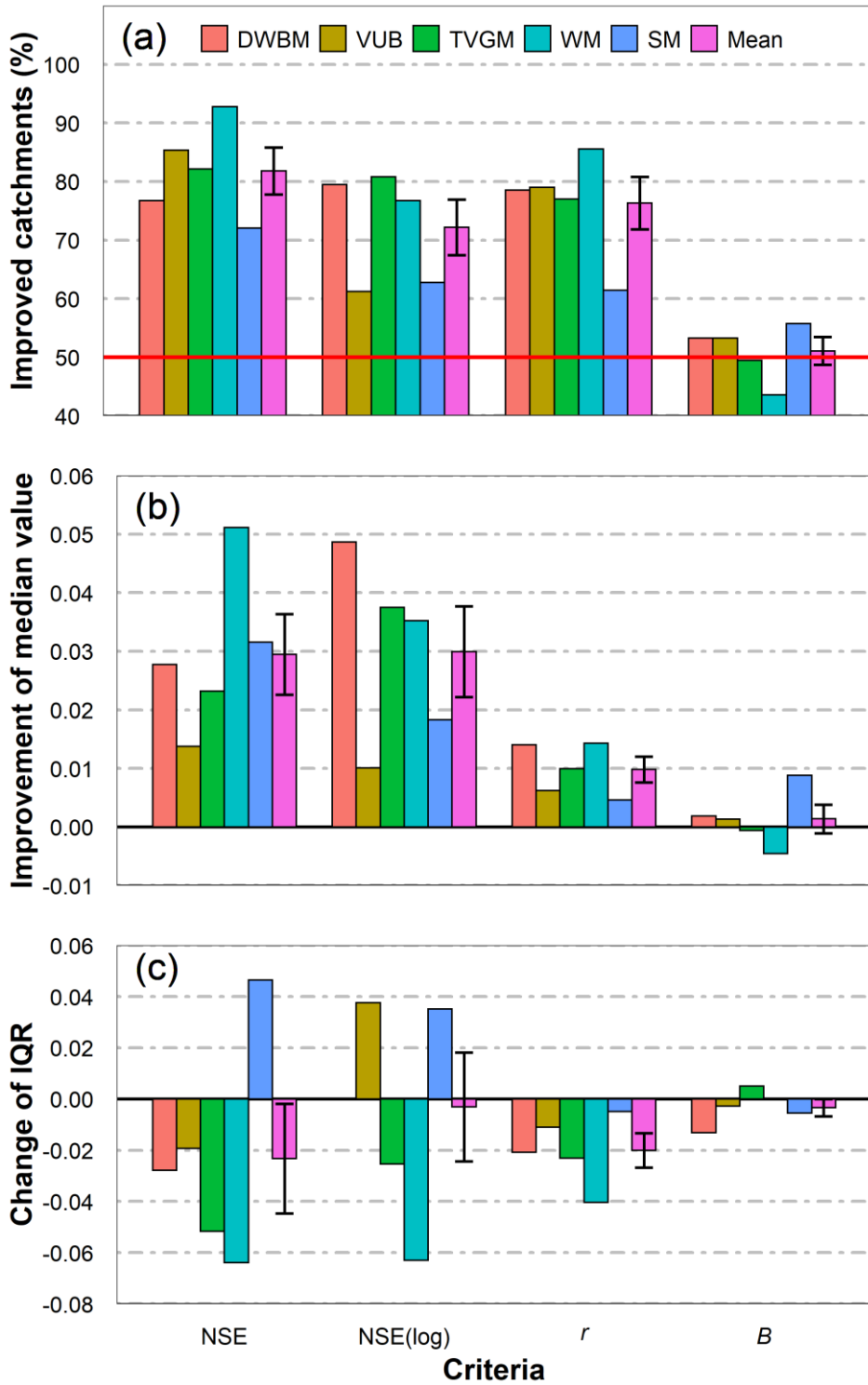


Figure 6. Comparison of total streamflow performance between original and modified models. (a) the percentage of improved catchments, (b) improvement of median value and, (c) change of IQR in terms of NSE, NSE (log), r and B of all the 443 catchments. The bar and error bar of the mean indicate mean and standard deviation of all the 5 models and all the 443 catchments.

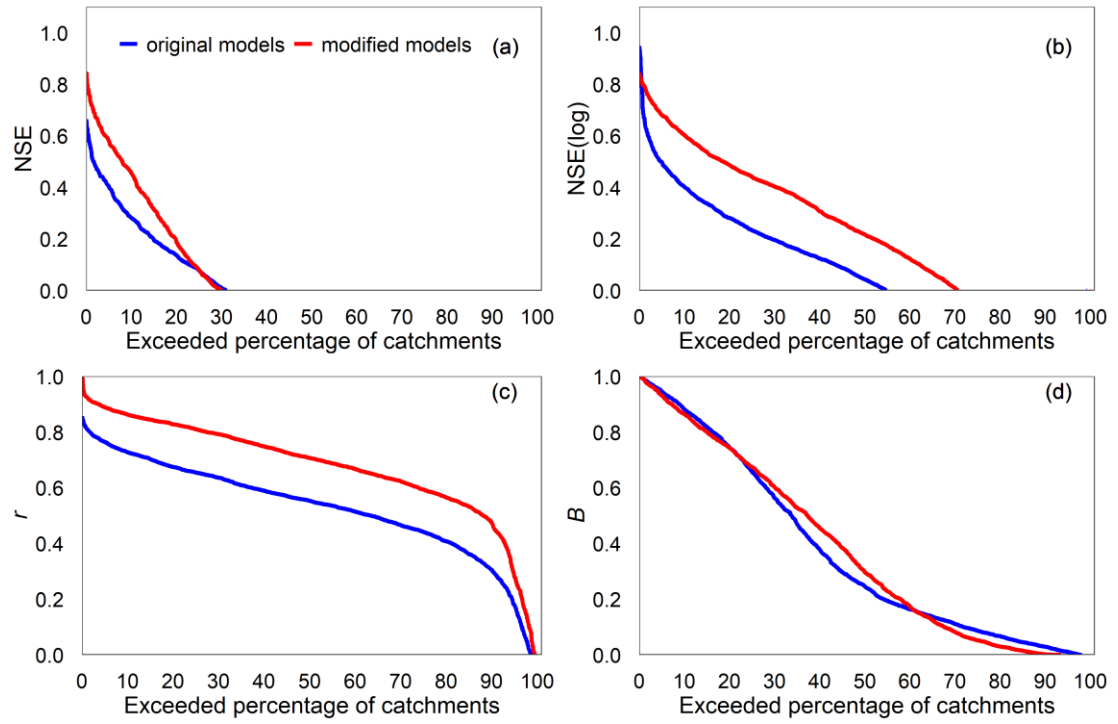


Figure 7. Same as Figure 5 except for baseflow.

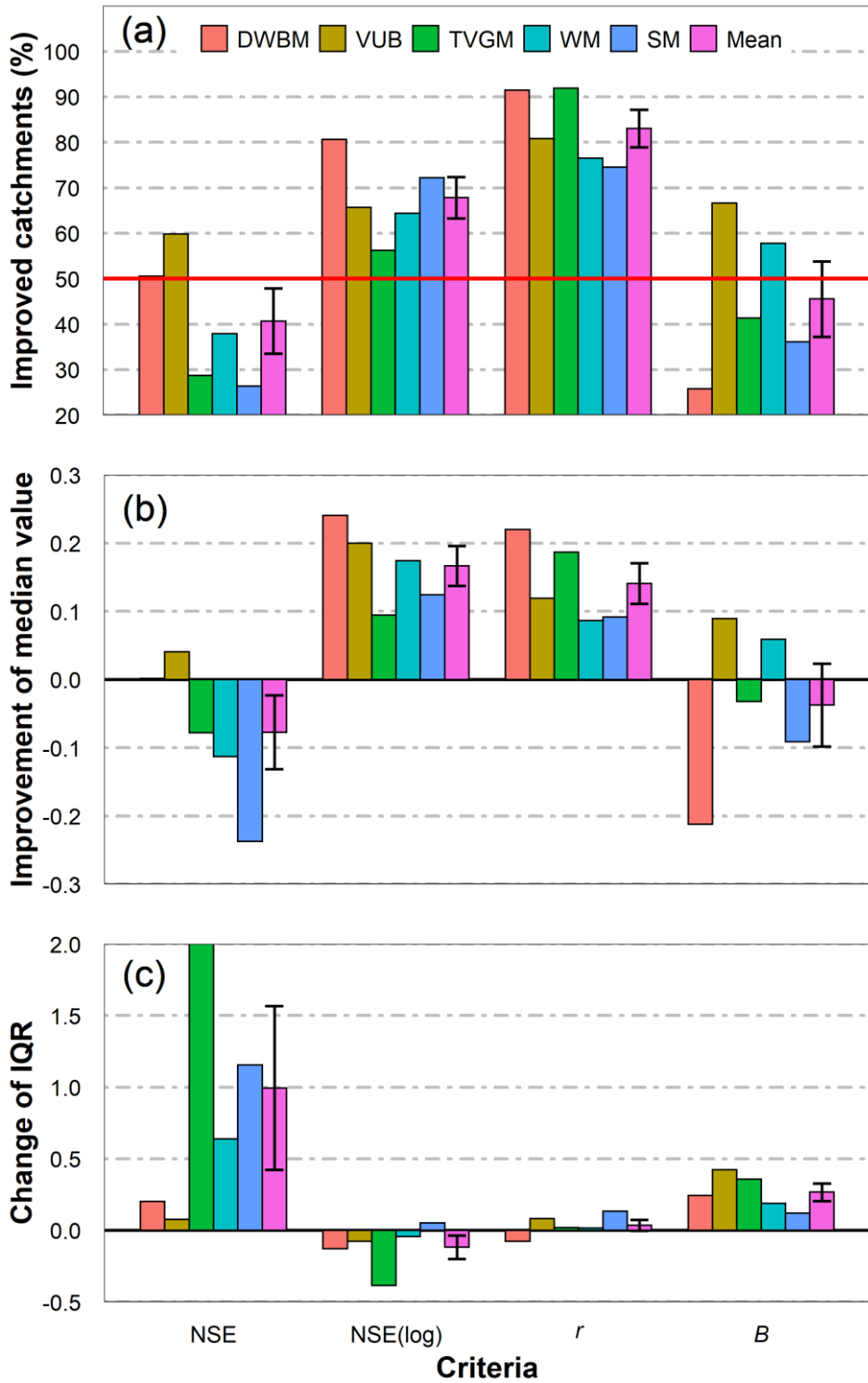


Figure 8. Same as Figure 6 except for baseflow. Note that maximum change of IQR of NSE of the WM model is 2.9 and the y-axis of subplot (c) is truncated to 2.0 for a better visualization.

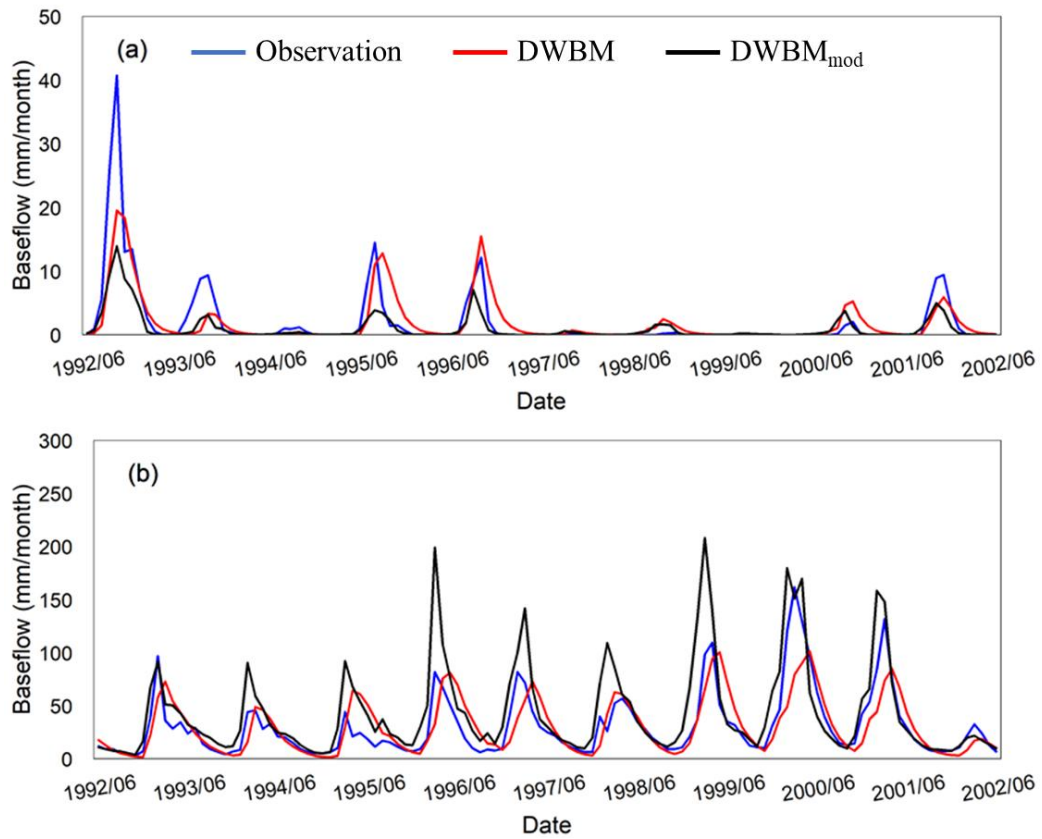


Figure 9. Time series of monthly baseflow (mm month^{-1}) from observation (blue line) and simulated by DWBM (red line) and DWBM_{mod} (black line) in two selected catchments: (a) 238204 and (b) 108002. Note only ten-year records are showed for a better visualization.

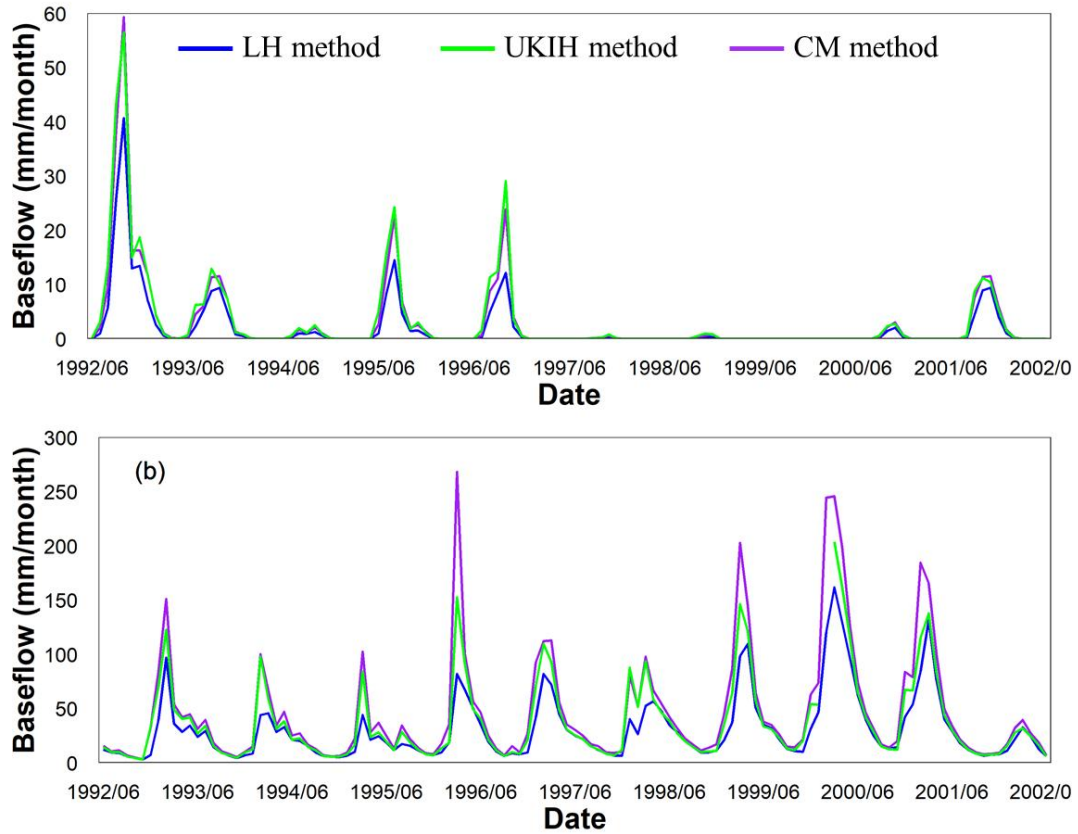


Figure 10. Comparison of baseflow derived from LH method (blue line), UKIH method (green line) and CM method (purple line) in catchments (a) 238204 and (b) 108002.

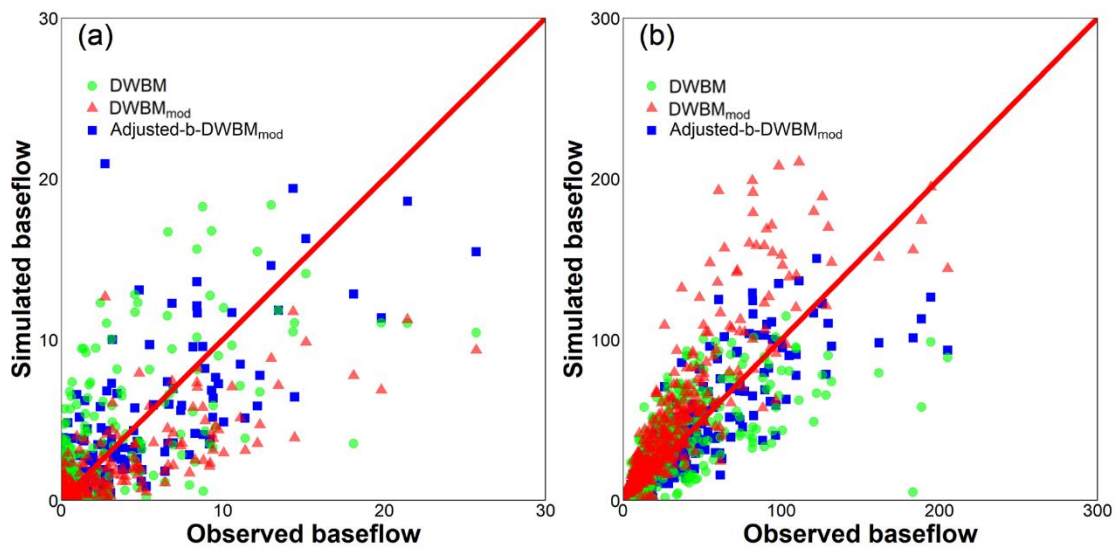


Figure 11. Scatter plots of observed and simulated monthly baseflow (mm month^{-1}) by DWBM (green dots), DWBM_{mod} (red triangles) and Adjusted- b - DWBM_{mod} (blue squares) in two selected catchments: (a) 238204 and (b) 108002.

Table 1. Summary of the catchment characteristics in the 443 catchments including tropics, arid, equiseasonal-hot, equiseasonal-warm and winter rainfall dominant.

Catchment characteristics	Total	Arid	Equiseasonal hot	Equiseasonal warm	Winter rainfall	Tropics
Number of catchments	443	50	105	171	61	56
Catchment area (km ²)	48-72902	65-72902	53-15851	51-16953	48-11795	66-47651
Mean annual rainfall (mm)	230-3684	230- 892	547-1791	491-2405	294-1129	760-3684
Mean annual potential evapotranspiration (mm)	921-2238	1214-1988	1190-1819	921-1495	1046-1553	1641-2238
Aridity index	0.39-6.99	2.21-6.99	0.76-2.69	0.39-2.31	1.14-5.28	0.48-2.49
Annual runoff coefficient	0.000-0.961	0.000-0.398	0.025-0.735	0.029-0.861	0.005-0.263	0.106-0.961
Annual baseflow index	0.001-0.792	0.001-0.027	0.032-0.509	0.033-0.792	0.062-0.799	0.061-0.605
CV of monthly precipitation	0.47-1.92	0.73-1.92	0.65-1.13	0.47-1.05	0.57-1.11	1.00-1.57
CV of monthly runoff	0.61-313.65	4.13-107.22	1.32-52.36	0.61-38.51	4.23-313.65	1.14-16.84

Table 2. Equations of the 5 models for simulating actual evaporation, quick flow and baseflow.

Model	Parameters	Equations to simulate actual evapotranspiration	No.	Equations to simulate quick flow	No.	Equations to simulate baseflow	No.
DWBM	S_{max}, a_1, a_2, d	$E_a(t) = W(t) \times F\left(\frac{PET(t)}{W(t)}, a_2\right)$	(w ₁)	$Q_d(t) = P(t) \times \left(1 - F\left(\frac{X_0(t)}{P(t)}, a_1\right)\right)$	(w ₂)	$Q_b(t) = dG(t - 1)$	(w ₃)
VUB	x_1, x_2, x_3	$E_a(t) = \min\left[PET(t) \times \left(1 - x_1 \frac{W(t)}{PET(t)}\right), W(t)\right]$	(w ₄)	$P_e(t) = P(t) - PET(t) \times \left(1 - e^{\frac{-P(t)}{PET(t)}}\right)$ $Q_d(t) = x_3 S(t - 1) \times P_e(t)$	(w ₅) (w ₆)	$Q_b(t) = x_2 S(t - 1)$	(w ₇)
TVGM	$g_1, g_2, k_r, S_{max}, \gamma$	$E_a(t) = PET(t) \times (S(t - 1)/S_{max})^\gamma$	(w ₈)	$Q_d(t) = g_1 (S(t - 1)/S_{max})^{g_2} \times P(t)$	(w ₉)	$Q_b(t) = k_r (S(t - 1) + S(t))/2$	(w ₁₀)
WM	S_{max}, k_s, k_g	$E_a(t) = PET(t) \times S(t - 1)/S_{max}$	(w ₁₁)	$Q_d(t) = k_s (S(t - 1)/S_{max}) \times P(t)$	(w ₁₂)	$Q_b(t) = k_g S(t - 1)$	(w ₁₃)
SM	$D_{max}, G_{max}, k, z, \theta$	$E_a(t) = PET(t) \times \frac{D_{max} - D(t)}{D_{max}}$	(w ₁₄)	$P_e(t) = P(t) - \theta E_a(t) - zD(t)$ $Q_d(t) = P_e(t)^2 / (P_e(t) + D_{max})$	(w ₁₅) (w ₁₆)	$Q_b(t) = k(G_{max} - D(t))$	(w ₁₇)

Table 3. Summary of the linear or nonlinear characteristics of actual evapotranspiration, quick flow and baseflow simulating equations of the 5 MWBMs.

Model	Actual evapotranspiration		Quick flow		Baseflow	
	linear	nonlinear	linear	nonlinear	linear	nonlinear
DWBM		✓		✓	✓	
VUB		✓		✓	✓	
TVGM		✓		✓	✓	
WM	✓			✓	✓	
SM	✓			✓	✓	

Table 4. The function for baseflow generation mechanism in the 5 original and modified models.

Original model	Equation for baseflow	Modified model	Equation for baseflow
DWBM	$Q_b(t) = dG(t - 1)$	DWBM _{mod}	$Q_b(t) = e^{(W(t)-b)/m}$
VUB	$Q_b(t) = x_2S(t - 1)$	VUB _{mod}	$Q_b(t) = e^{(W(t)-b)/m}$
TVGM	$Q_b(t) = k_r (S(t - 1) + S(t))/2$	TVGM _{mod}	$Q_b(t) = e^{((S(t-1)+S(t))/2-b)/m}$
WM	$Q_b(t) = k_gS(t - 1)$	WM _{mod}	$Q_b(t) = e^{(W(t)-b)/m}$
SM	$Q_b(t) = k(G_{max} - D(t))$	SM _{mod}	$Q_b(t) = e^{(D_{max}-D(t)+P(t)-b)/m}$
Note:	$W(t) = S(t - 1) + P(t)$		

Table 5. The value of F_{avg} at 25th, 50th and 75th percentile across 443 catchments of total streamflow (Q_t), quick flow (Q_d) and baseflow (Q_b) simulated by five original models. The IQR is inter-quantile range (*i.e.* range between 75th and the 25th percentiles). The row of “Average” means the average value of F_{avg} of the five models.

Model	F_{avg} of total streamflow				F_{avg} of quick flow				F_{avg} of baseflow			
	25th	50th	75th	IQR	25th	50th	75th	IQR	25th	50th	75th	IQR
DWBM	0.72	0.77	0.84	0.12	0.41	0.56	0.68	0.27	0.09	0.30	0.48	0.39
VUB	0.72	0.77	0.83	0.11	0.52	0.63	0.69	0.17	0.00	0.12	0.21	0.21
TVGM	0.61	0.71	0.8	0.19	0.47	0.55	0.62	0.15	-0.14	0.11	0.19	0.33
WM	0.58	0.68	0.76	0.18	0.44	0.52	0.6	0.16	-0.21	0.12	0.21	0.42
SM	0.63	0.71	0.77	0.14	0.47	0.57	0.66	0.19	0.24	0.39	0.51	0.27
Average	0.65	0.73	0.80	0.15	0.46	0.57	0.65	0.19	0.00	0.21	0.32	0.32

Table 6. Summary of the improved values of different indicators for the total streamflow performance comparing the modified and original models. The last row shows the average (mean \pm standard deviation) of all the 5 models.

Model	NSE			NSE(log)			<i>r</i>			<i>B</i>		
	Proportion (%)	Median	IQR	Proportion (%)	Median	IQR	Proportion (%)	Median	IQR	Proportion (%)	Median	IQR
DWBM	76.75	0.03	-0.06	79.46	0.05	-0.06	78.56	0.01	-0.04	53.27	0.00	0.00
VUB	85.33	0.01	0.05	61.17	0.01	0.04	79.01	0.01	0.00	53.27	0.00	-0.01
TVGM	82.17	0.02	-0.02	80.81	0.04	0.04	76.98	0.01	-0.01	49.44	0.00	0.00
WM	92.78	0.05	-0.03	76.75	0.04	0.00	85.55	0.01	-0.02	43.57	0.00	-0.01
SM	72.01	0.03	-0.05	62.75	0.02	-0.03	61.40	0.00	-0.02	55.76	0.01	0.00
Range	72.01~92.78	0.01~0.05	-0.06~0.05	61.17~80.81	0.01~0.05	-0.06~0.04	61.40~85.55	0.00~0.01	-0.04~0.00	43.57~55.76	0.00~0.01	-0.01~0.00
Average	81.81 \pm 3.99	0.03 \pm 0.007	-0.02 \pm 0.02	72.19 \pm 4.73	0.03 \pm 0.008	-0.002 \pm 0.02	76.30 \pm 4.48	0.01 \pm 0.002	-0.02 \pm 0.01	51.06 \pm 2.38	0.002 \pm 0.002	-0.004 \pm 0.003

Table 7. Same as Table 6 except for baseflow.

Model	NSE			NSE (log)			<i>r</i>			<i>B</i>		
	Proportion (%)	Median	IQR	Proportion (%)	Median	IQR	Proportion (%)	Median	IQR	Proportion (%)	Median	IQR
DWBM	50.56	0.00	0.64	80.59	0.24	-0.05	91.42	0.22	0.02	25.73	-0.21	0.19
TVGM	28.67	-0.08	0.08	56.21	0.09	-0.08	91.87	0.19	0.08	41.31	-0.03	0.42
VUB	59.82	0.04	1.15	65.69	0.20	0.05	80.81	0.12	0.13	66.59	0.09	0.12
WM	37.92	-0.11	0.20	64.33	0.17	-0.13	76.52	0.09	-0.08	57.79	0.06	0.24
SM	26.41	-0.24	2.90	72.23	0.12	-0.38	74.49	0.09	0.02	36.12	-0.09	0.36
Range	26.41~59.82	-0.24~0.04	0.08~2.90	56.21~80.59	0.09~0.24	-0.38~0.05	74.49~91.42	0.09~0.22	-0.08~0.13	25.73~66.59	-0.21~0.09	0.12~0.42
Average	40.68±7.16	-0.08±0.05	0.99±0.57	67.81±4.57	0.17±0.03	-0.12±0.08	83.02±4.10	0.14±0.03	0.03±0.04	45.51±8.26	-0.04±0.06	0.27±0.06

Table 8. Summary of model parameters and performances of the DWBM, DWBM_{mod} and Adjusted-*b*-DWBM_{mod} in catchment 238204 and 108002.

Station	Model	Parameters		Criteria			
		<i>m</i>	<i>b</i>	<i>r</i>	NSE	NSE(log)	<i>B</i>
238204	DWBM	/	/	0.713	0.471	0.126	0.811
	DWBM _{mod}	47.5	233.9	0.840	0.572	0.343	0.612
	Adjusted- <i>b</i> - DWBM _{mod}	47.5	210	0.840	0.705	0.491	0.988
108002	DWBM	/	/	0.698	0.468	0.259	0.998
	DWBM _{mod}	226.8	1.73	0.872	0.307	0.657	0.427
	Adjusted- <i>b</i> - DWBM _{mod}	226.8	100	0.872	0.753	0.772	0.925